

GENERAL NOTES

- 1. ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE FOLLOWING CODES:
2. THE INTERNATIONAL BUILDING CODE (IBC) 2012 AND ITS REFERENCED STANDARDS, HEREIN REFERRED TO AS "THE CODE" AND OTHER REGULATORY CRITERIA WHICH HAVE AUTHORITY OVER ANY PORTION OF THE WORK.
3. PRIOR TO FABRICATION AND CONSTRUCTION, THE CONTRACTOR SHALL VERIFY EXISTING ELEVATIONS AND DIMENSIONS ASSOCIATED WITH THE WORK. ALL OMISSIONS OR CONFLICTS BETWEEN VARIOUS ELEMENTS OF THE CONTRACT DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE ENGINEER OF RECORD PRIOR TO PROCEEDING WITH THE RELATED WORK.
4. CONTRACTOR SHALL INVESTIGATE SITE DURING CLEARING AND EARTHWORK OPERATIONS FOR FILLED EXCAVATIONS OR BURIED STRUCTURES, SUCH AS FOUNDATIONS, ETC. THE ENGINEER OF RECORD SHALL BE NOTIFIED IMMEDIATELY IF ANY SUCH STRUCTURES ARE FOUND.
5. THE STRUCTURAL DRAWINGS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN LIVE LOADS.
6. THESE CONTRACT DRAWINGS WERE PREPARED WITH THE ASSISTANCE OF OWNER PROVIDED INFORMATION. THE CONTRACTOR IS RESPONSIBLE FOR BECOMING COMPLETELY FAMILIAR WITH ALL EXISTING CONDITIONS AND VERIFICATION OF EXISTING CONSTRUCTION, ELEVATIONS, AND DIMENSIONS. IF EXISTING CONDITIONS VARY FROM THE REQUIREMENTS OF THE CONTRACT, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE OWNERS REPRESENTATIVE BEFORE WORK STARTS.

STRUCTURAL DESIGN DATA

LIVE LOADS:

- LOBBIES, EXERCISE ROOMS, GATHERING ROOMS:
40PSF
FIRST FLOOR CORRIDORS:
100 PSF
SECOND FLOOR CORRIDORS:
80 PSF
OFFICES, EXAM ROOMS, BATHROOMS, LOCKER ROOMS, MISCELLANEOUS ROOMS:
50 PSF AND 20 PSF PARTITION
MECHANICAL ROOMS
125 PSF UNLESS INDIVIDUAL MECHANICAL EQUIPMENT GOVERNS

SNOW LOADS: IN ACCORDANCE WITH THE REQUIREMENTS OF THE CODE, AS MODIFIED BY CLIENT REQUEST BASED ON "SNOW COVER IN ALASKA: COMPREHENSIVE REVIEW" BY GIENKO ET AL. 2018.

BUILDING

- Po = 186 lb/ft²
Pr = 130 lb/ft²
Co = 1.0
Ci = 1.0
I = 1
Ps = 130 lb/ft²

CANOPIES

- Pc = 156 lb/ft²
Co = 1.0
Ci = 1.2
Ps = 156 lb/ft²

WIND LOADS: IN ACCORDANCE WITH THE CODE.

- BASIC WIND SPEED V = 145 MPH
WIND IMPORTANCE I = 1.00
OCCUPANCY CATEGORY II
WIND EXPOSURE CATEGORY EXPOSURE B
INTERNAL PRESSURE COEFFICIENT

Gcpi = + 0.18 (MAIN BUILDING)

COMPONENT AND CLADDING WIND LOADS BUILDING

Table with columns: ZONE, 10 ft², 20 ft², 50 ft², 100 ft². Rows include MAIN ROOF, EDGE ROOF, CORNER ROOF, MAIN WALL, EDGE WALL, ROOF OVERHANG.

COMPONENT AND CLADDING WIND LOADS CANOPY

Table with columns: ZONE, < 13 ft², < 52 ft², > 52 ft². Rows include MAIN ROOF, EDGE ROOF, CORNER ROOF.

SEISMIC LOADS: BASED ON THE EQUIVALENT LATERAL FORCE PROCEDURE IN ACCORDANCE WITH THE REQUIREMENTS OF THE CODE.

- SEISMIC IMPORTANCE FACTOR I = 1.0
OCCUPANCY CATEGORY II
SITE CLASS D
SHORT-PERIOD DESIGN ACCELERATION Sps = 1.09g
1-SECOND DESIGN ACCELERATION Sd1 = 0.76g
SEISMIC DESIGN CATEGORY D
RESPONSE MODIFICATION FACTOR

- R = 6 1/2 (LIGHT FRAMED WOOD CONSTRUCTION SHEATHED WITH STRUCTURAL PANELS)

- R = 3 1/2 (ORDINARY STEEL MOMENT FRAMES)

ARCHITECTURAL, MECHANICAL, AND ELECTRICAL COMPONENTS

- 1. ALL COMPONENTS SHALL BE ANCHORED TO THE BUILDING STRUCTURE. ANCHORAGE SHALL BE DESIGNED FOR ALL DESIGN CASES, INCLUDING SEISMIC, BY THE CONTRACTOR'S ENGINEER AND SUBMITTED TO THE ENGINEER FOR APPROVAL. DRAWINGS AND CALCULATIONS SHALL BE SEALED BY A REGISTERED ENGINEER IN THE STATE OF ALASKA.

STRUCTURAL CONCRETE NOTES

- 1. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO CHP 19 OF THE CODE AND THE PROVISIONS IN ACI 318.
2. SUITABLE CONCRETE MIXES SHALL BE PREPARED BY A QUALIFIED TESTING LABORATORY AND APPROVED BY THE ENGINEER OF RECORD. CONCRETE SPECIFIED BY COMPRESSIVE STRENGTH SHALL BE PROPORTIONED ON THE BASIS DESCRIBED IN 1905.1.1 OF THE CODE.
3. SCHEDULE OF CAST-IN-PLACE CONCRETE 28 DAY COMPRESSIVE STRENGTHS AND TYPES:

Table with columns: CONDITION, STRENGTH (PSI), DENSITY (PCF), W/C RATIO, AIR ENTRAINMENT. Rows include FOUNDATIONS, SLAB-ON-GRADES.

- 4. PORTLAND CEMENT SHALL CONFORM TO ASTM STANDARD C-150 AND TYPE AS FOLLOWS:
A. TYPE I/III - TYPICAL USE IN WARM/COLD SEASON CONCRETE, RESPECTIVELY.
B. TYPE II/V - FOR USE IN MODERATE/HIGH SULFATE CORROSIVE SOILS.
5. AGGREGATE FOR HARD-ROCK CONCRETE (150 PCF) SHALL CONFORM TO THE REQUIREMENTS AND TESTS OF ASTM C-33.
6. ALL CONCRETE PERMANENTLY EXPOSED TO THE WEATHER SHALL CONTAIN AN APPROVED AIR-ENTRAINING ADMIXTURE IN CONFORMANCE WITH ASTM C-260.
7. ALL REINFORCING BARS SHALL BE DEFORMED BAR CONFORMING TO THE STANDARDS OF ASTM A615, GRADE 60.
8. ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS OF THE LATEST EDITION OF CHP 19 OF THE CODE, ACI 318 AND THE "ACI DETAILING MANUAL: DETAILS AND DETAILING CONCRETE REINFORCEMENT", ACI 315.
9. CHECKED SHOP DRAWINGS SHOWING REINFORCING DETAILS, INCLUDING STEEL SIZES, SPACING AND PLACEMENT SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW PRIOR TO FABRICATION.
10. REINFORCING BAR SPLICES SHALL BE MADE AS INDICATED ON THE DRAWINGS. LAP ALL HORIZONTAL BARS AT CORNERS AND INTERSECTIONS. STAGGER ALL SPLICES UNLESS NOTED OTHERWISE ON PLANS.
11. DESIGN, REMOVAL AND RESHORING OF FORMWORK SHALL BE IN ACCORDANCE WITH ACI 318, CHP 6.
12. WHERE REQUIRED, DOWELS SHALL MATCH SIZE AND NUMBER OF MAIN REINFORCING.

- 13. MAXIMUM SLUMP SHALL BE 4 INCHES, UNO.
14. MINIMUM CONCRETE COVER SHALL BE:
A. 3" FOR CONCRETE CAST AGAINST THE EARTH.
B. 1 1/2" FOR BARS EXPOSED TO WEATHER AND BEAMS AND COLUMNS.
C. 3/4" FOR SLABS.
15. FOR COLD-WEATHER PLACEMENT (WHEN TEMPERATURE IS EXPECTED TO FALL BELOW 40 DEGREES F FOR THREE CONSECUTIVE DAYS), COMPLY WITH ACI 306.1 DO NOT USE FROZEN MATERIALS, MATERIALS CONTAINING ICE OR SNOW, OR CALCIUM CHLORIDE, SALT, OR OTHER MATERIALS CONTAINING ANTIFREEZE AGENTS OR CHEMICAL ACCELERATORS. A TEMPERATURE OF 50 DEGREES F MUST BE MAINTAINED DURING CURING VIA USE OF TENTING OR OTHER ACCEPTABLE ENCLOSURES. CONCRETE (OTHER THAN HIGH-EARLY-STRENGTH) SHALL BE MAINTAINED ABOVE 50 DEGREES F AND IN A MOIST CONDITION FOR FOR AT LEAST THE FIRST 7 DAYS AFTER PLACEMENT. HIGH-EARLY-STRENGTH CONCRETE SHALL BE MAINTAINED ABOVE 50 DEGREES F AND IN A MOIST CONDITION FOR AT LEAST THE FIRST 3 DAYS.

FOUNDATION NOTES

- 1. FOUNDATION DESIGN IS BASED ON THE SOILS REPORT BY NORTHERN GEOTEHNNICAL / TERRA FIRMA, DATED 12/13/2016.
2. FOUNDATIONS & WALLS ARE DESIGNED BASED ON THE FOLLOWING INFORMATION:
A. ALLOWABLE BEARING PRESSURE \* : 3,000 PSF
a. VALUES MAY BE INCREASED BY 1/3 FOR WIND OR SEISMIC LOAD CASES.
b. VALUES MAY BE INCREASED BY 300 PSF FOR EACH HORIZONTAL FOOT OF INCREASE ABOVE 2'-0" WITH A MAXIMUM BEARING PRESSURE OF 5,300 PSF.

B. COEFFICIENT OF FRICTION = 0.40

- 3. ALL FOOTING SUBGRADES AS REQUIRED AND ALL SLAB SUBGRADES INCLUDING PIT SLABS SHALL BE COMPACTED TO 95 PERCENT OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT. ALL BACKFILL AROUND AND ABOVE ALL FOUNDATION ELEMENTS, FOOTINGS, CAPS, MATS, WALLS AND PITS SHALL BE COMPACTED TO 95 PERCENT OF MAXIMUM DENSITY.
4. ALL ORGANIC AND/OR OTHER UNSUITABLE MATERIALS SHALL BE REMOVED FROM SUBGRADE AND BACKFILL AREAS AND BACKFILLED WITH ACCEPTABLE GRANULAR FILL, COMPACTED TO 95 PERCENT OF MAXIMUM DENSITY.
5. CONTRACTOR SHALL PROVIDE FOR DESIGN AND INSTALLATION OF ALL CRIBBING, SHEATHING AND SHORING REQUIRED AND SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH ALL NATIONAL, STATE AND LOCAL SAFETY ORDINANCES.
6. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY MEASURES TO PREVENT ANY FROST OR ICE FROM PENETRATING ANY FOOTING OR SLAB SUBGRADES BEFORE AND AFTER PLACING OF CONCRETE UNTIL SUCH SUBGRADES ARE FULLY PROTECTED BY THE PERMANENT BUILDING STRUCTURE.
7. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS BEFORE CONCRETE OR GROUT HAS ATTAINED FULL DESIGN STRENGTH. CONTRACTORS SHALL BRACE OR PROTECT ALL BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHING FLOORS ARE COMPLETELY IN PLACE AND HAVE ATTAINED FULL STRENGTH. CONTRACTOR SHALL PROVIDE FOR DESIGN, PERMITS AND INSTALLATION OF SUCH BRACING.
8. THE CONCRETE FOR EACH ISOLATED FOOTING SHALL BE PLACED IN ONE (1) CONTINUOUS PLACEMENT.
9. NO CONSTRUCTION SHALL COMMENCE UNTIL ALL SEASONAL FROST HAS THAWED OR BEEN REMOVED.

WOOD TRUSSES

- 1. WOOD TRUSSES SHALL BE FACTORY BUILT AND SHALL CONFORM WITH THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE AND DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES PUBLISHED BY THE TRUSS PLATE INSTITUTE.
2. ENGINEERING DESIGN AND SHOP DRAWINGS BEARING THE STAMP OF AN ENGINEER REGISTERED IN THE STATE OF ALASKA AND SHOWING ALL DETAILS OF CONSTRUCTION SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL.

STRUCTURAL WOOD NOTES

- 1. ALL STRUCTURAL LUMBER SHALL BE VISUALLY OR MACHINE STRESS GRADED, IN ACCORDANCE WITH THE LATEST EDITIONS OF THE WEST COAST LUMBER INSPECTION BUREAU (WCLIB) TECHNICAL PUBLICATION NO. 17 OR THE WESTERN WOOD PRODUCTS ASSOCIATION (WWPA) "WESTERN LUMBER GRADING RULES (G5)". THE DESIGN AND CONSTRUCTION STANDARDS OF ALL WOOD FRAMING SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 23 OF THE CODE AND THE LATEST EDITION OF THE "AMERICAN FOREST & PAPER ASSOCIATION NATIONAL DESIGN SPECIFICATION".
2. ALL CONVENTIONAL AND COMPOSITE FRAMING MATERIALS SHALL CONFORM TO THE FOLLOWING:
A. CONVENTIONAL SAWN LUMBER ≤ 5 INCH SQ:
a. SPECIES: HEM FIR
b. GRADE: NO. 1
B. CONVENTIONAL SAWN LUMBER > 5 INCH SQ:
a. SPECIES: HEM FIR
b. GRADE: NO. 1
C. GLU-LAM LUMBER:
a. SPECIES/GRADE: DF/HF 24F-V4 (SIMPLE SPAN)
b. SPECIES/GRADE: DF/HF 24F-V8 (MULTI-SPAN / CANOPY)
c. MODULUS OF ELASTICITY: 1,800,000 PSI
D. PLYWOOD SHEATHING:
a. ALL STRUCTURAL PLYWOOD SHEATHING SHALL BE DOUGLAS FIR STANDARD GRADE STRUCTURAL I WITH EXTERIOR GLUE CONFORMING TO THE LATEST EDITION OF PS 1.
b. ALL STRUCTURAL COMPOSITE SHEATHING (OSB) SHALL BE DOUGLAS FIR STANDARD GRADE STRUCTURAL I WITH EXTERIOR GLUE CONFORMING TO THE LATEST EDITION OF PS 2.
c. ALL PANELS SHALL BEAR LEGIBLE APA STAMPS.
3. INSTALL ALL PLYWOOD WITH THE LONG DIMENSION OF THE PANEL ACROSS SUPPORTS, UNLESS NOTED OTHERWISE, WITH THE PANEL OVER TWO OR MORE SPANS. ALLOW 1/8 INCH SPACING AT PANEL ENDS AND 1/4 INCH AT PANEL EDGES, UNLESS OTHERWISE RECOMMENDED BY THE PANEL MANUFACTURER. PLYWOOD SHALL BE USED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN PLYWOOD ASSOCIATION.
4. ALL PLYWOOD FLOOR PANELS SHALL BE GLUE-NAILED TO FLOOR FRAMING PER THE PLANS. USE ONLY ADHESIVES CONFORMING TO APA SPECIFICATION AFG-01, APPLIED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. IF NON VENEER PANELS WITH SEALED SURFACES AND EDGES ARE TO BE USED, USE ONLY SOLVENT-BASED GLUES; CHECK WITH PANEL MANUFACTURER.
5. THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL TIMBER MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT THE PRIOR REVIEW OF THE ENGINEER.
6. ALL NAILS SHALL BE COMMON WIRE NAILS. NAILING SHALL CONFORM TO TABLE 2304.9.1 OF THE CODE.
7. STANDARD WASHERS SHALL BE USED UNDER ALL BOLT HEADS AND NUTS CONTACTING WOOD.
8. IF PNEUMATIC NAILERS ARE TO BE USED THE CONTRACTOR MUST SUBMIT A SCHEDULE OF FASTENERS AS DESIRED AS A SUBSTITUTION TO THE ENGINEER FOR APPROVAL.
9. NO WOOD TREATMENTS OR PRESERVATIVES SHALL BE USED WITHOUT PRIOR REVIEW OF THE ENGINEER.
10. ALL WOOD LEDGERS, PLATES, SILLS, AND NAILERS IN CONTACT WITH CONCRETE, EARTH, OR WITHIN 6" OF EARTH SHALL BE TREATED IN ACCORDANCE SECTION 2303.1.8 OF THE CODE. NAILS AND METAL FASTENERS SHALL BE GALVANIZED OR STAINLESS STEEL AS RECOMMENDED BY THE TREATED WOOD MANUFACTURER.
11. ALL OTHER WOOD CONSTRUCTION IDENTIFIED IN THE CODE SECTION 2304.11 SHALL BE PRESSURE-TREATED.
12. ALL BELOW GRADE FASTENERS SHALL BE TYPE 304 OR TYPE 316 STAINLESS STEEL FASTENERS.
13. ALL FASTENERS IN CONTACT WITH PRESSURE-TREATED WOOD SHALL BE HOT-DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE, OR COPPER FASTENERS.

Architects Alaska logo and contact information for ARCHITECTS ALASKA AK Corp.

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Professional Engineer seal for ANIL KESHAV MESTAS, State of Alaska, No. 42919, Exp. 12/31/2024.

YCHC ALTERNATE DESIGN FOUNDATION PACKAGE logo for YAKUTAT TLINGIT TRIBE, YAKUTAT, ALASKA.

Revisions table with columns: No., Description, Date. Row 1: REVISIONS, 06/18/19.

Checked by AKM, Job No. 17341JN, Date 04/29/2019.

Sheet Contents GENERAL NOTES (1 OF 2)

Sheet No. S0.1

**STRUCTURAL STEEL NOTES**

- ALL STRUCTURAL STEEL SHALL BE CONSISTENT WITH THE FOLLOWING STANDARDS:  
 STRUCTURAL WF SHAPES ..... ASTM A992  
 STRUCTURAL HSS TUBES ..... ASTM A500, GRADE C  
 STRUCTURAL STEEL PIPE ..... ASTM A53, GRADE B  
 STEEL PLATES & MISC ..... ASTM A36
- ALL BOLTS, NUTS AND WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325, UNLESS NOTED OTHERWISE. ALL BOLTS SHALL BE 3/4 INCH DIAMETER, UNO.
- ALL WELDING ELECTRODES SHALL BE E70XX.
- ALL DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO AISC SPECIFICATIONS AND CODES, LATEST EDITION.
- ALL WELDING SHALL BE DONE BY QUALIFIED WELDERS AND SHALL CONFORM TO THE AWS "D1.1 STRUCTURAL WELDING CODE-STEEL", LATEST EDITION.
- THE FABRICATOR/ERECTOR SHALL SUBMIT TO THE ENGINEER, FOR REVIEW, ENGINEERED AND CHECKED DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS AND ERECTION DIAGRAMS FOR ALL STRUCTURAL STEEL.
- ALL CONNECTIONS SHALL BE SIMPLE SHEAR CONNECTIONS USING HIGH-STRENGTH BOLTS IN BEARING TYPE CONNECTIONS WITH THREADS EXCLUDED FROM THE SHEAR PLANE IN SINGLE SHEAR, UNO.
- WHERE BOLTED CONNECTION ARE NOT REQUIRED BY DESIGN THE CONTRACTOR SHALL PROVIDE A MINIMUM OF (2) BOLTS PER CONNECTION.
- ALL BEAMS, JOISTS AND TRUSSES SHALL BE FABRICATED AND ERECTED WITH THE REQUIRED CAMBER UP. PROVIDE CAMBERS AS INDICATED ON THE DRAWINGS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF ALL ERECTION PROCEDURES AND SEQUENCES WITH RELATION TO TEMPERATURE DIFFERENTIALS, ESPECIALLY WITH RESPECT TO STRUCTURAL STEEL FRAMING INTO CONCRETE WALLS, BEAMS OR COLUMNS.
- THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT PRIOR APPROVAL OF THE ENGINEER.
- STEEL PAINTING: ALL STEEL SHALL BE CLEANED BY METHODS COMPLYING WITH THE STEEL STRUCTURES PAINTING COUNCIL. REMOVE OIL, GREASE, AND SIMILAR CONTAMINANTS. EXCEPT FOR MEMBERS TO BE WELDED, APPLY STRUCTURAL STEEL PRIMER PAINT IN ACCORDANCE WITH THE MANUFACTURERS INSTRUCTIONS AT A RATE TO PROVIDE A UNIFORM DRY FILM THICKNESS OF 2.0 MILS. AFTER FINAL STEEL INSTALLATION, WIRE BRUSH EXPOSED STEEL SURFACES AND CLEAN WITH SOLVENTS BEFORE TOUCH-UP PAINTING. TOUCH-UP PAINT SHALL BE THE SAME AS SHOP PAINT.
- ALL EXTERIOR STEEL SHALL BE HOT DIPPED GALVANIZED.
- ALL EXTERIOR BOLTS SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL.
- THE CONTRACTOR SHALL PROVIDE SHOP DRAWINGS OF ALL STEEL MEMBERS, PLATES AND CONNECTION HARDWARE INCLUDING COATING. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.

**ABBREVIATIONS**

- @ ..... AT
- AB ..... ANCHOR BOLT
- AFF ..... ABOVE FINISH FLOOR
- AISC ..... AMERICAN INSTITUTE OF STEEL CONST.
- APPROX ..... APPROXIMATELY
- ARCH ..... ARCHITECTURAL
- AWW ..... ALL WEATHER WOOD
- BCI ..... BOISE CASCADE I-JOIST
- BET/BTWN ..... BETWEEN
- BM ..... BEAM
- BN ..... BOUNDARY NAILING
- BOF ..... BOTTOM OF FOOTING
- BOS ..... BOTTOM OF STEEL
- BOT ..... BOTTOM
- BS ..... BOTH SIDES
- CFT ..... CONCRETE FILLED TUBE
- CJ ..... CONTROL JOINT
- CONC ..... CONCRETE
- CONT ..... CONTINUOUS
- CONTR'S ..... CONTRACTORS
- CJP ..... COMPLETE JOINT PENETRATION
- DIA/DIAM/ø ..... DIAMETER
- DICA ..... DRILLED IN CONCRETE ANCHOR
- DIM ..... DIMENSION
- DL ..... DEAD LOAD
- DWG ..... DRAWING
- (E) ..... EXISTING
- EA ..... EACH
- EJ ..... EXPANSION JOINT
- ELEV ..... ELEVATION
- EMBED ..... EMBEDDED
- EOR ..... ENGINEER OF RECORD
- EQ ..... EQUAL
- EW ..... EACH WAY
- FDN ..... FOUNDATION
- FF ..... FINISH FLOOR
- FTG ..... FOOTING
- GA ..... GAGE
- GLB ..... GLULAM BEAM
- H/HORIZ ..... HORIZONTAL
- IBC ..... INTERNATIONAL BUILDING CODE
- INSUL ..... INSULATION
- JT ..... JOINT
- LL ..... LIVE LOAD
- LOC ..... LOCATION
- LONG ..... LONGITUDINAL
- MANUF ..... MANUFACTURER
- MATL ..... MATERIAL
- MECH ..... MECHANICAL
- MIN ..... MINIMUM
- NIC ..... NOT IN CONTRACT
- NTS ..... NOT TO SCALE
- OC ..... ON CENTER
- OPP ..... OPPOSITE HAND
- PL ..... PLATE
- REF ..... REFERENCE
- REINF ..... REINFORCEMENT
- REQ'D ..... REQUIRED
- SCHED ..... SCHEDULE
- SIM ..... SIMILAR
- SOG ..... SLAB ON GRADE
- SQ ..... SQUARE
- STD ..... STANDARD
- STL ..... STEEL
- T&B ..... TOP & BOTTOM
- TO ..... TOP OF
- TOC ..... TOP OF CONCRETE
- TOS ..... TOP OF STEEL
- TYP ..... TYPICAL
- UNO ..... UNLESS NOTED OTHERWISE
- V/VERT ..... VERTICAL
- W/ ..... WITH
- WP ..... WORKING POINT
- WWF ..... WELDED WIRE FABRIC



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**YAKUTAT TLINGIT TRIBE**  
**YCHC ALTERNATE DESIGN**  
**FOUNDATION PACKAGE**  
**YAKUTAT, ALASKA**

Revisions		
No.	Description	Date
1	REVISIONS	06/18/19

Drawn by DJM	Date 04/29/2019
Checked AKM	Job No. 17341JN

Sheet Contents  
 GENERAL NOTES (2 OF 2)

Sheet No.  
**S0.2**

TABLE 1 - SPECIAL INSPECTION SCHEDULE  
REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

Table with 6 columns: REQUIRED VERIFICATION AND INSPECTION, QA, QC, REFERENCE STANDARD \*, RESPONSIBLE CHARGE, IBC REFERENCE. Rows include inspection tasks prior to, during, and after welding and bolting, with various reference standards like AISC 360.

TABLE 2 - SPECIAL INSPECTION SCHEDULE  
REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

Table with 6 columns: VERIFICATION AND INSPECTION, CONTINUOUS, PERIODIC, REFERENCE STANDARD \*, RESPONSIBLE CHARGE, IBC REFERENCE. Rows include inspection of reinforcing steel, bolts, and concrete placement.

TABLE 3 - SPECIAL INSPECTION SCHEDULE  
SPECIAL INSPECTION FOR STEEL SEISMIC RESISTANCE

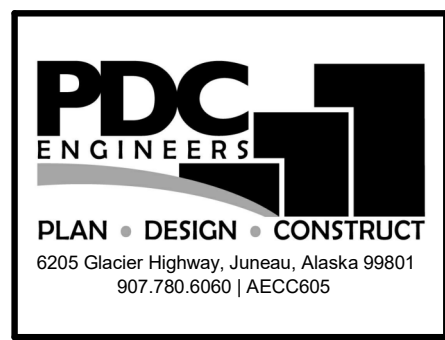
Table with 7 columns: VERIFICATION AND INSPECTION, QC (TASK, DOC), QA (TASK, DOC), REFERENCE STANDARD, RESPONSIBLE CHARGE, IBC REFERENCE. Rows include visual inspection tasks prior to and during welding, and inspection tasks after welding and bolting.

TABLE 4 - SPECIAL INSPECTION SCHEDULE  
REQUIRED VERIFICATION AND INSPECTION OF WOOD CONSTRUCTION

Table with 6 columns: VERIFICATION AND INSPECTION, CONTINUOUS, PERIODIC, REFERENCE STANDARD \*, RESPONSIBLE CHARGE, IBC REFERENCE. Row includes nailing, bolting, and anchoring of components within the seismic force resisting system.

TABLE 5 - SPECIAL INSPECTION SCHEDULE  
REQUIRED VERIFICATION AND INSPECTION OF SOILS

Table with 6 columns: VERIFICATION AND INSPECTION, CONTINUOUS, PERIODIC, REFERENCE STANDARD, RESPONSIBLE CHARGE, IBC REFERENCE. Rows include verifying materials below shallow foundations and excavation depths.



YAKUTAT TLINGIT TRIBE  
YCHC ALTERNATE DESIGN  
FOUNDATION PACKAGE  
YAKUTAT, ALASKA

Revisions table with columns: No., Description, Date. Row 1: REVISIONS, 06/18/19.

Drawn by: DJM, Date: 04/29/2019. Checked by: AKM, Job No.: 17341JN.

Sheet Contents  
SPECIAL INSPECTIONS

Sheet No.  
S0.3

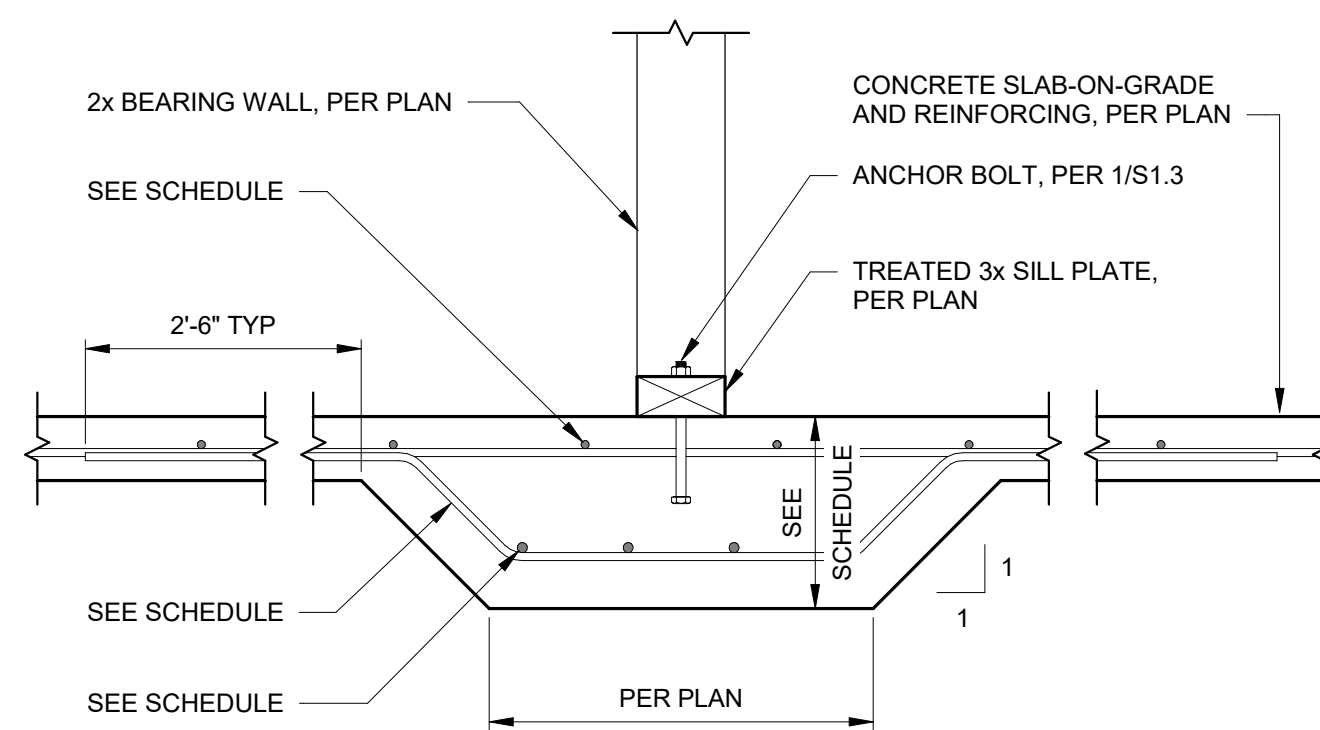
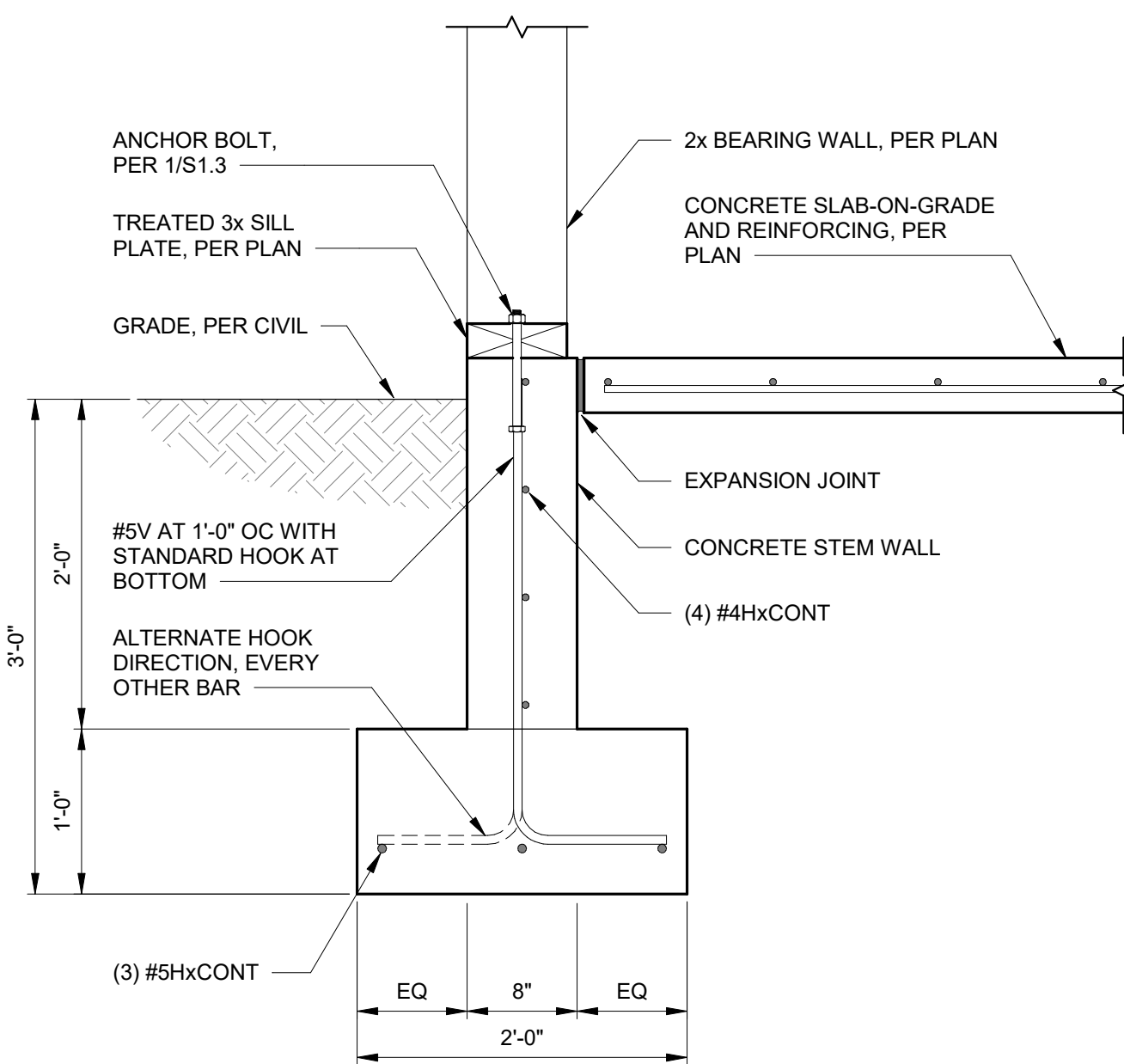
F'c (PSI)	CLASS "B" TENSION LAP SPLICE									
	BAR SIZE (GR 60)	3	4	5	6	7	8	9	10	11
4000	BAR DIAMETER (IN)	0.375	0.5	0.625	0.75	0.875	1	1.128	1.27	1.41
	TOP BAR	25	33	41	49	57	65	73	82	91
4500	BOTTOM BAR	19	25	31	37	44	50	56	63	70
	TOP BAR	24	31	39	47	54	62	69	78	86
	BOTTOM BAR	18	24	30	36	42	48	53	60	67

**SCHEDULE NOTES:**

- REINFORCEMENT CLEAR SPACING OF THE BARS BEING DEVELOPED OR SPLICED IS NOT LESS THAN ONE BAR DIAMETER, CLEAR COVER IS NOT LESS THAN ONE BAR DIAMETER AND STIRRUPS ARE PLACED CONTINUOUSLY THROUGHOUT SPLICE LENGTH.
- THE ABOVE VALUES ARE EXPRESSED FOR NORMAL-WEIGHT CONCRETE ONLY.
- THE ABOVE VALUES RELATE ONLY TO PLAIN (UNCOATED) DEFORMED REINFORCING.
- TOP BARS ARE HORIZONTAL REINFORCEMENT WITH MORE THAN 12" OF NEW CONCRETE PLACED MONOLTHICALLY BELOW BAR.
- BOTTOM BARS ARE HORIZONTAL REINFORCEMENT WITH LESS THAN 12" OF NEW CONCRETE PLACED BELOW BAR.

**1 REINFORCING LAP SCHEDULE**

S1.1 SCALE: 12" = 1'-0"



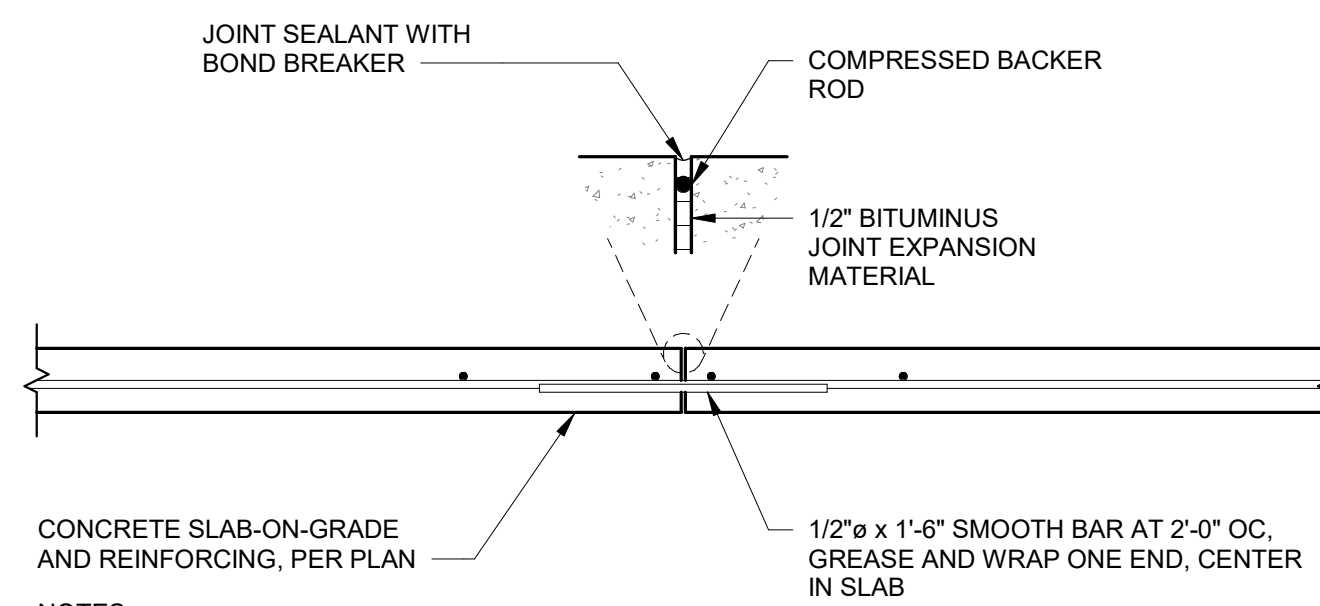
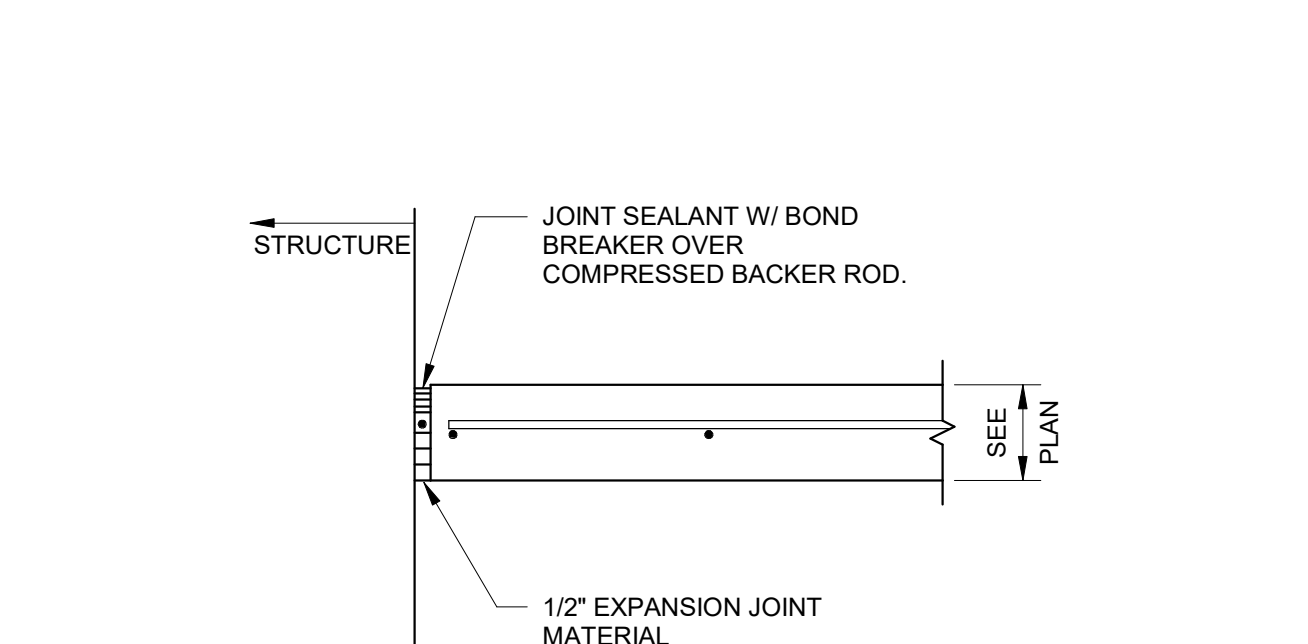
WIDTH (PER PLAN)	THICKNESS	REINFORCING			
		TOP		BOTTOM	
1'-6"	1'-0"	SLAB REINF	SLAB REINF	(3) #5HxCONT	#4H AT 1'-0" OC
2'-6"	1'-0"	SLAB REINF	SLAB REINF	(3) #5HxCONT	#4H AT 1'-0" OC
3'-6"	1'-8"	(5) #5HxCONT	#5H AT 10" OC	(6) #6HxCONT	#5H AT 10" OC

**5 TYPICAL THICKENED SLAB FOOTING**

S1.1 SCALE: 1" = 1'-0"

**4 TYPICAL EXTERIOR STEM WALL SECTION**

S1.1 SCALE: 1" = 1'-0"



- CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE ENGINEER OF RECORD.

**10 TYPICAL CONSTRUCTION JOINT**

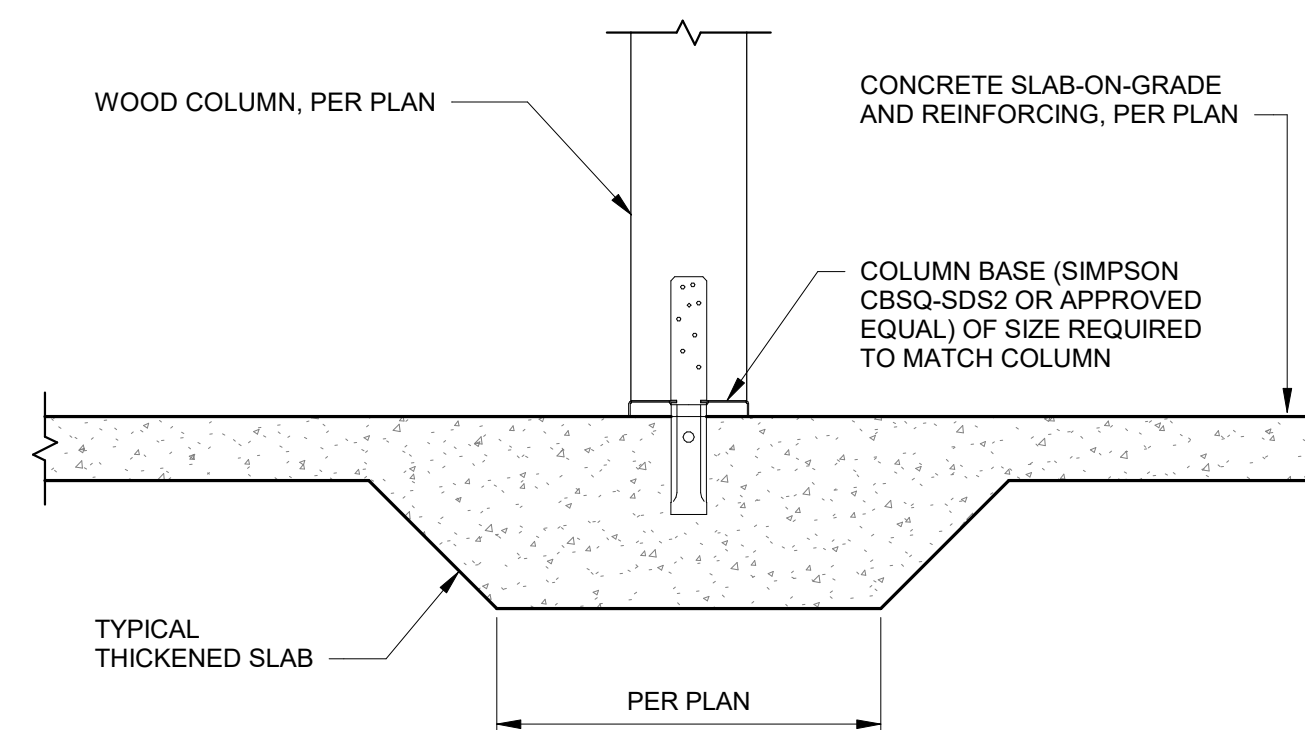
S1.1 SCALE: 1" = 1'-0"

**9 TYPICAL EXPANSION JOINT**

S1.1 SCALE: 1" = 1'-0"

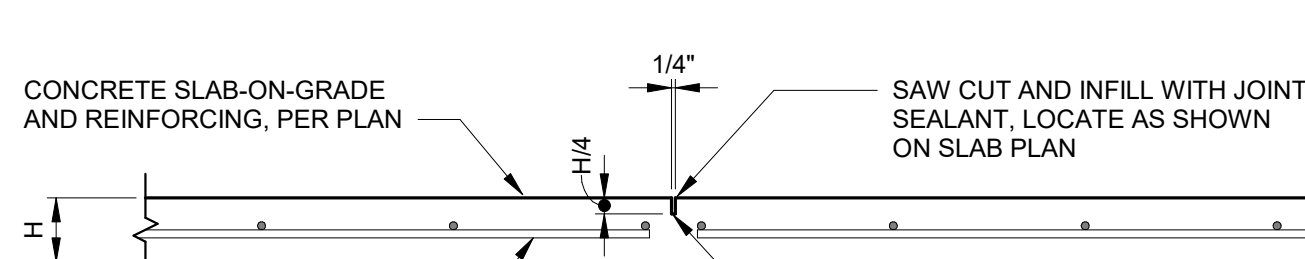
**2 TYPICAL REINF HOOK DETAIL**

S1.1 SCALE: 3/4" = 1'-0"



**6 TYPICAL WOOD COLUMN BASE**

S1.1 SCALE: 1" = 1'-0"

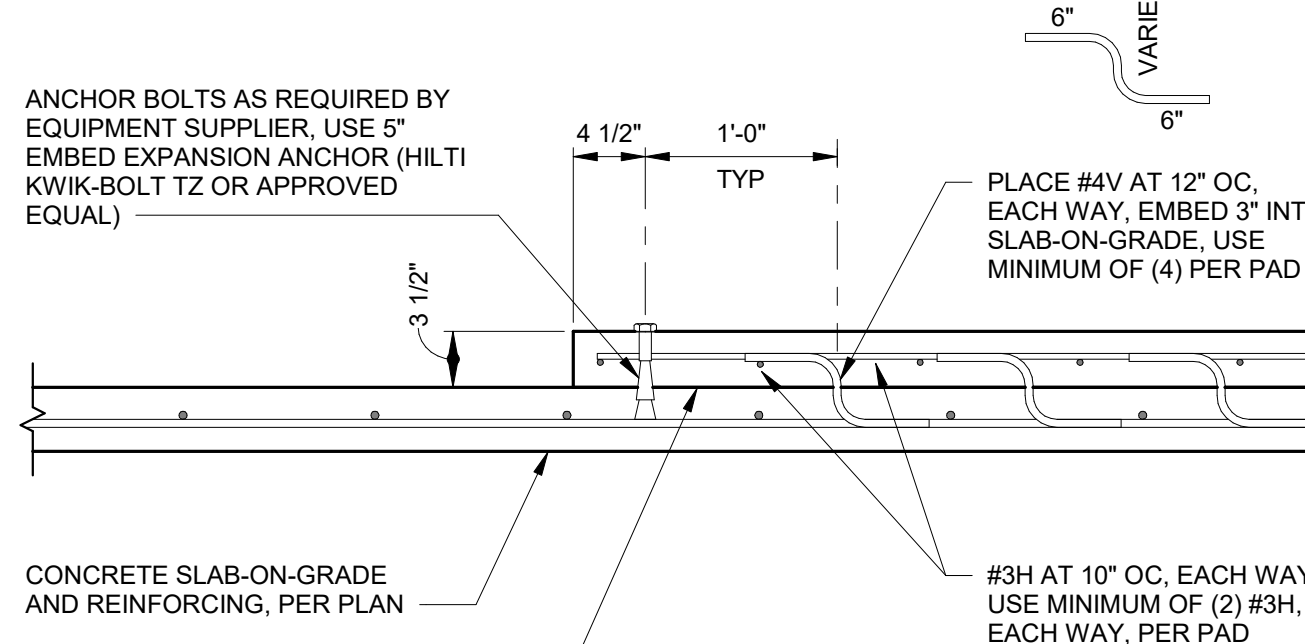


STOP EVERY OTHER BAR CROSSING CONTROL JOINT. PRE-MOLDED JOINT MATERIAL MAY BE SUBSTITUTED FOR SAW CUT.

- SAWED JOINTS SHALL BE MADE AS SOON AS THE JOINT CAN BE CUT WITHOUT EDGES RAVELING AND WITHIN 24 HOURS OF SLAB PLACEMENT.

**8 TYPICAL CONTROL JOINT**

S1.1 SCALE: 1" = 1'-0"



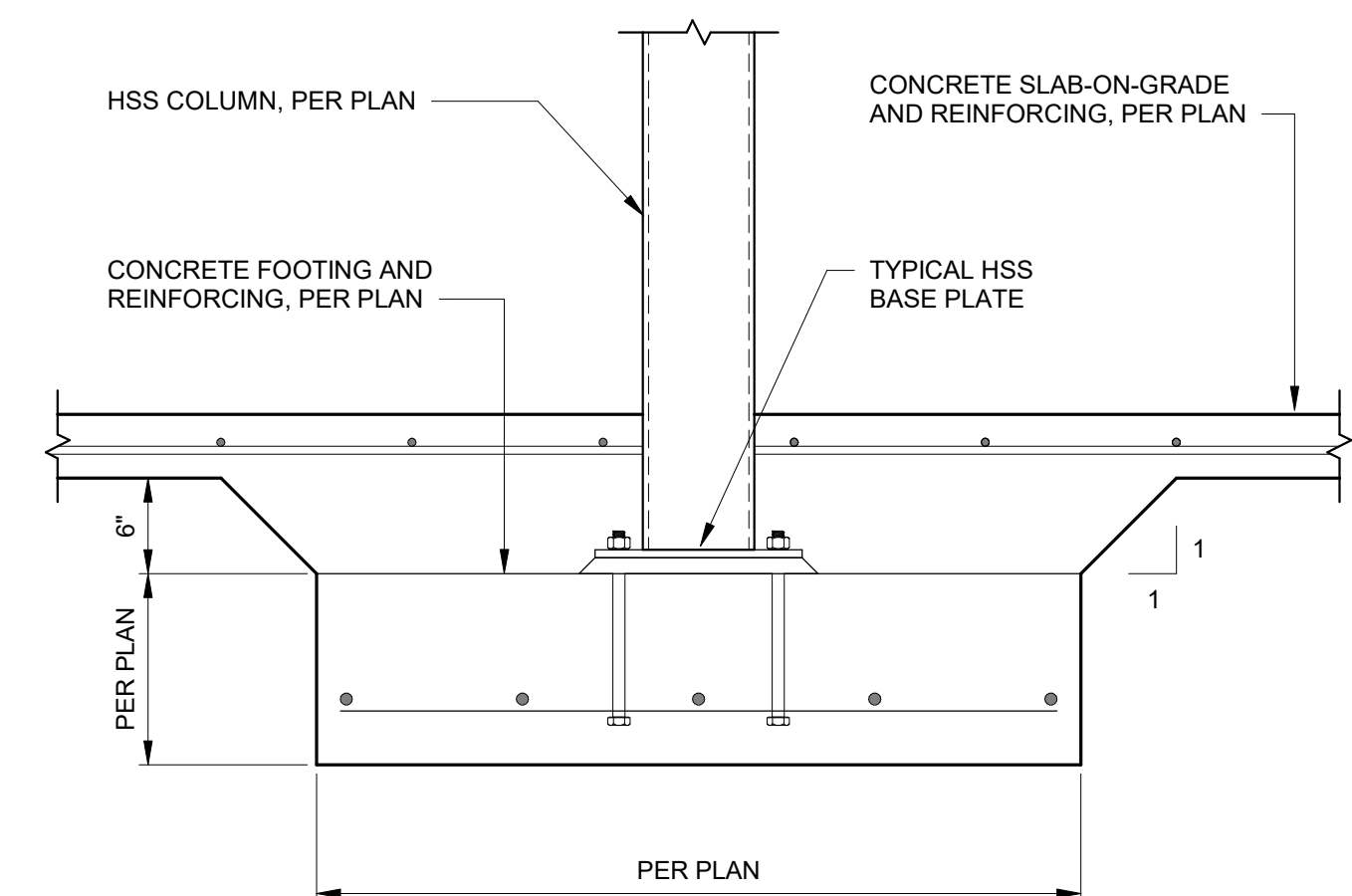
- USE BONDING COMPOUND BETWEEN CONCRETE SURFACES.

**11 TYPICAL HOUSEKEEPING PAD**

S1.1 SCALE: 1" = 1'-0"

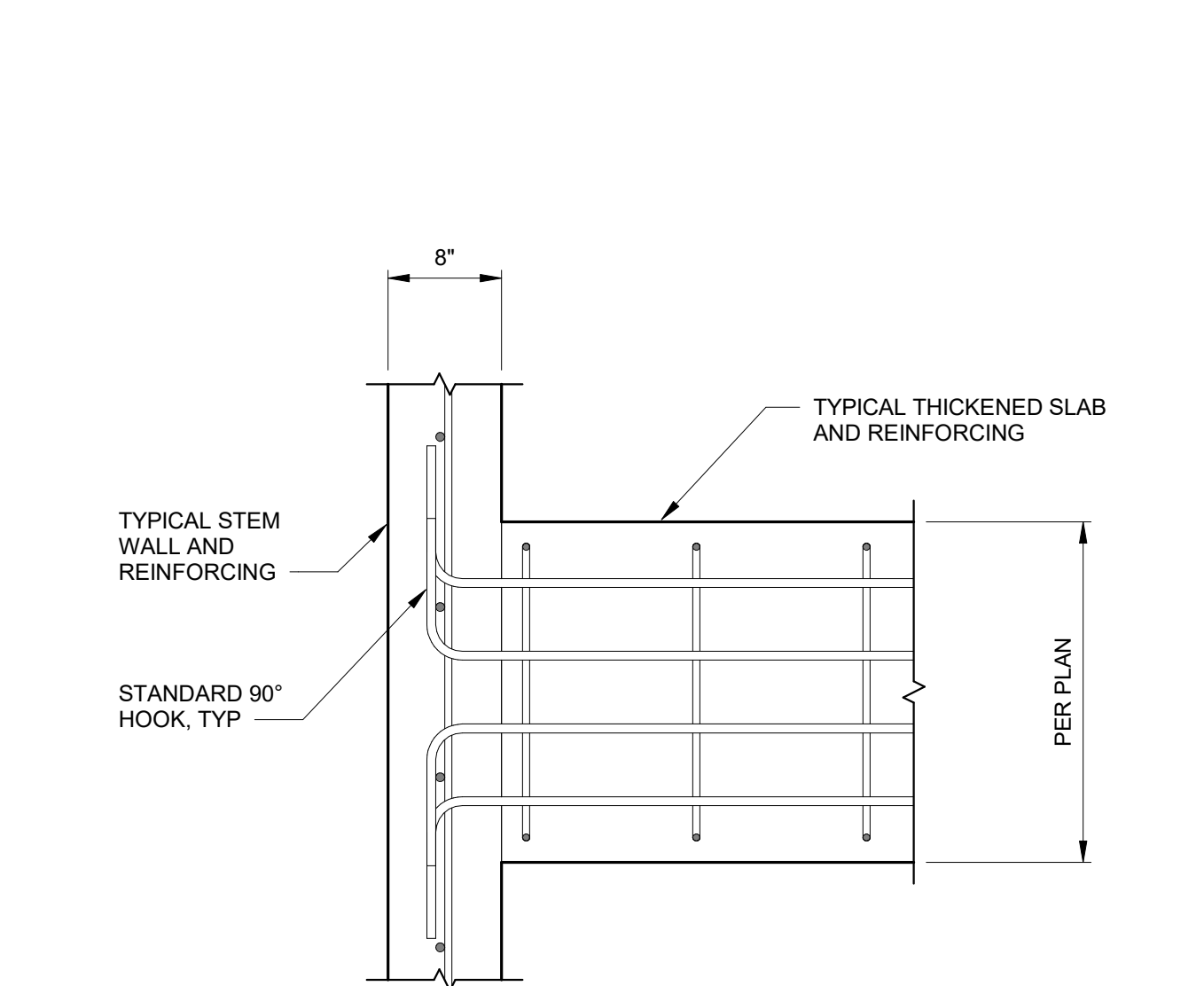
**3 TYPICAL INTERSECTION REINF**

S1.1 SCALE: 1" = 1'-0"



**7 TYPICAL INTERIOR FOOTING**

S1.1 SCALE: 1" = 1'-0"



**12 TYPICAL THICKENED SLAB TO STEM WALL**

S1.1 SCALE: 1" = 1'-0"

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 FOUNDATION PACKAGE  
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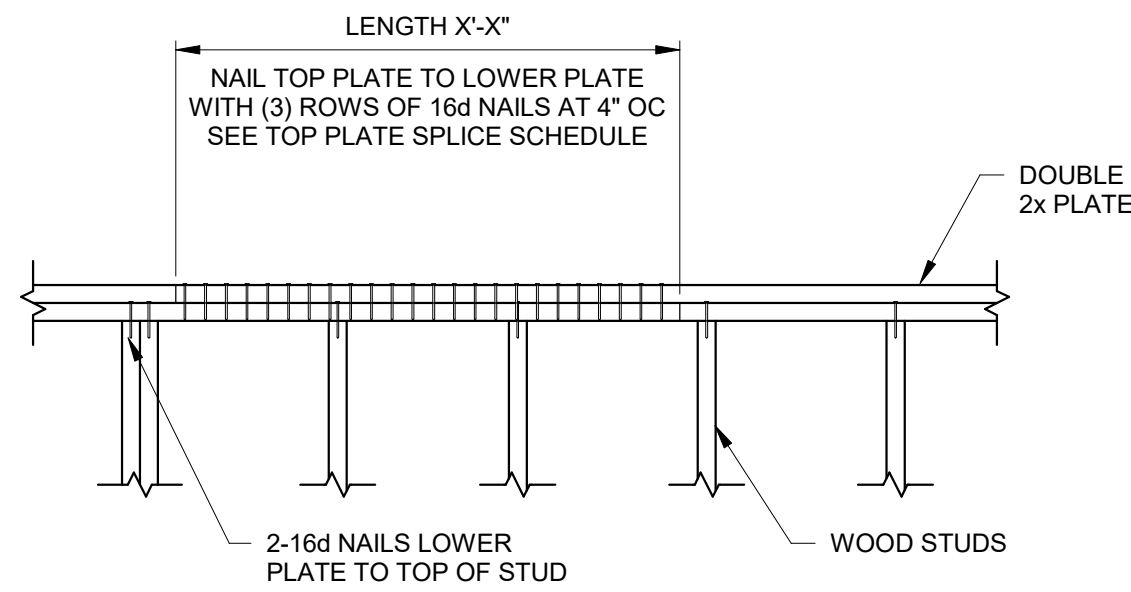
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Sheet Contents  
 TYPICAL FOUNDATION DETAILS

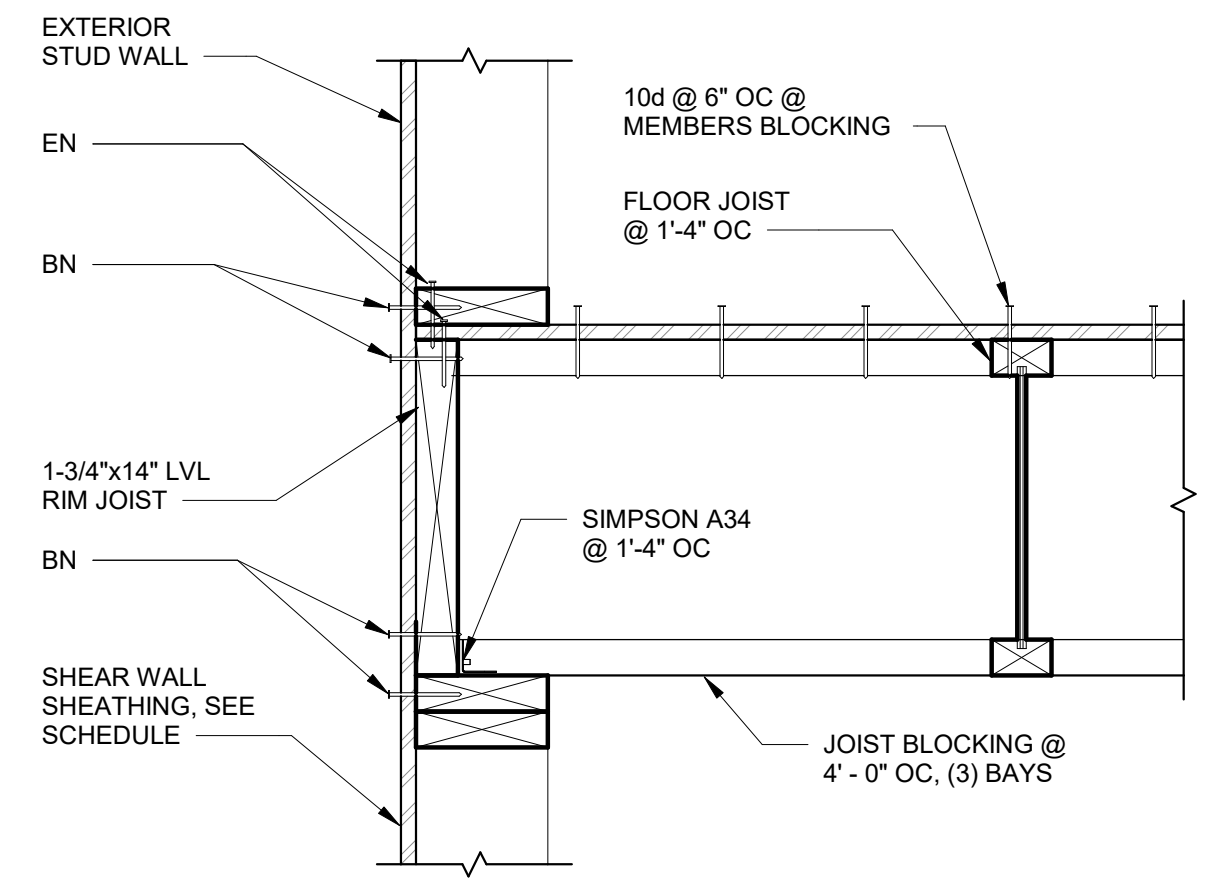
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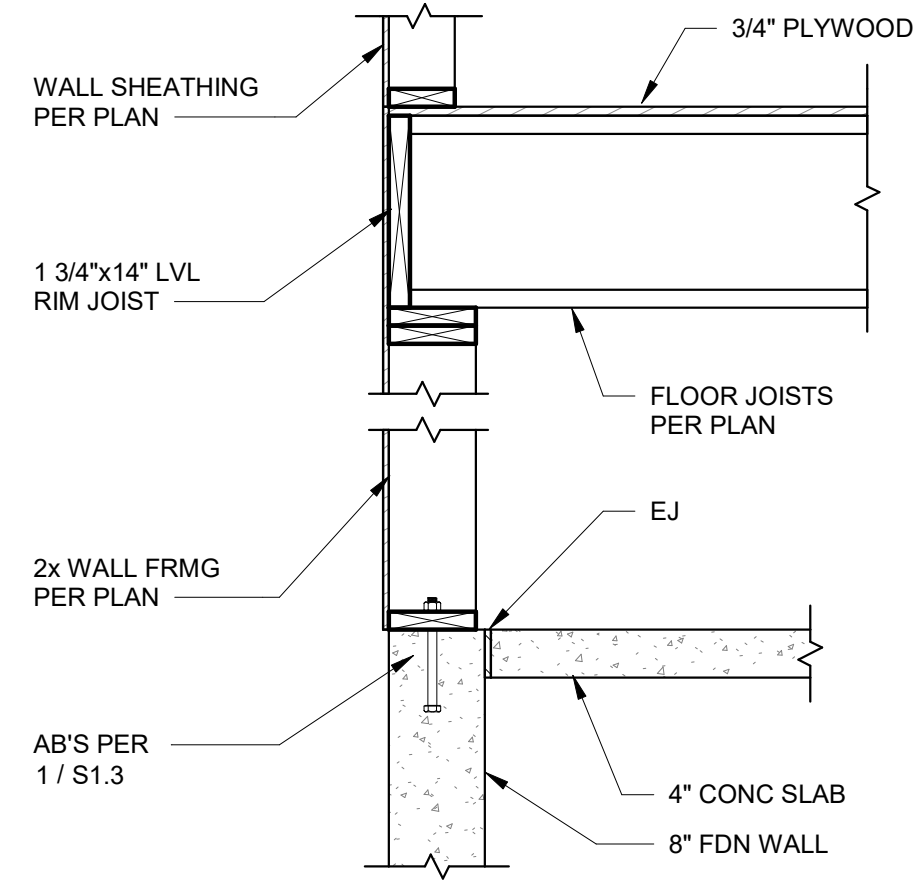


TOP PLATE SPLICE SCHEDULE		
ZONE	LENGTH X'-X"	TOTAL NAILS
ROOF	5'-6"	(45)
SECOND FLOOR	4'-0"	(24)

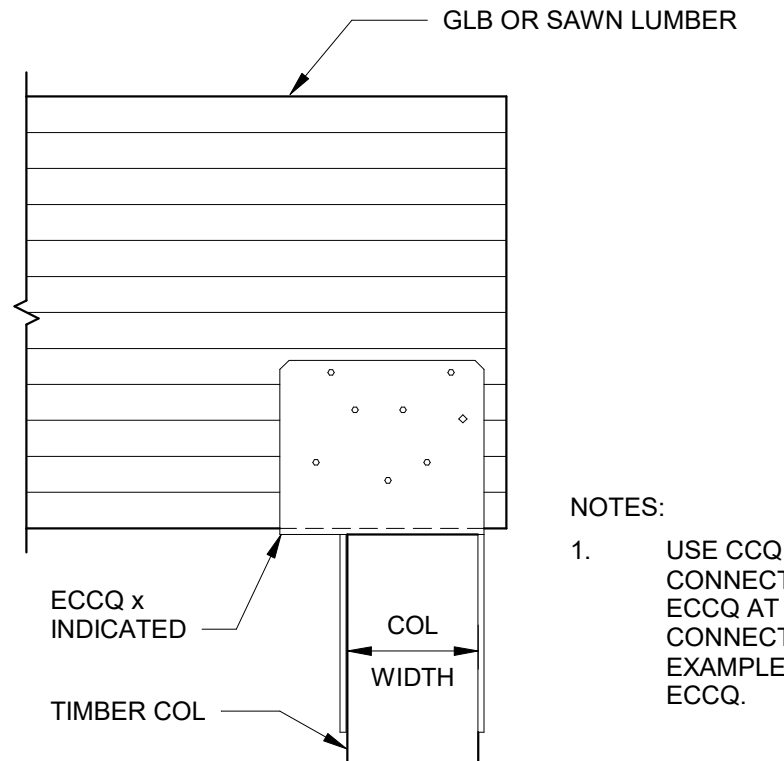
**1** TYPICAL TOP PLATE SPLICE  
S1.2 SCALE: 3/4" = 1'-0"



**2** SECOND FLOOR DIAPHRAGM CONNECTION  
S1.2 SCALE: 1 1/2" = 1'-0"



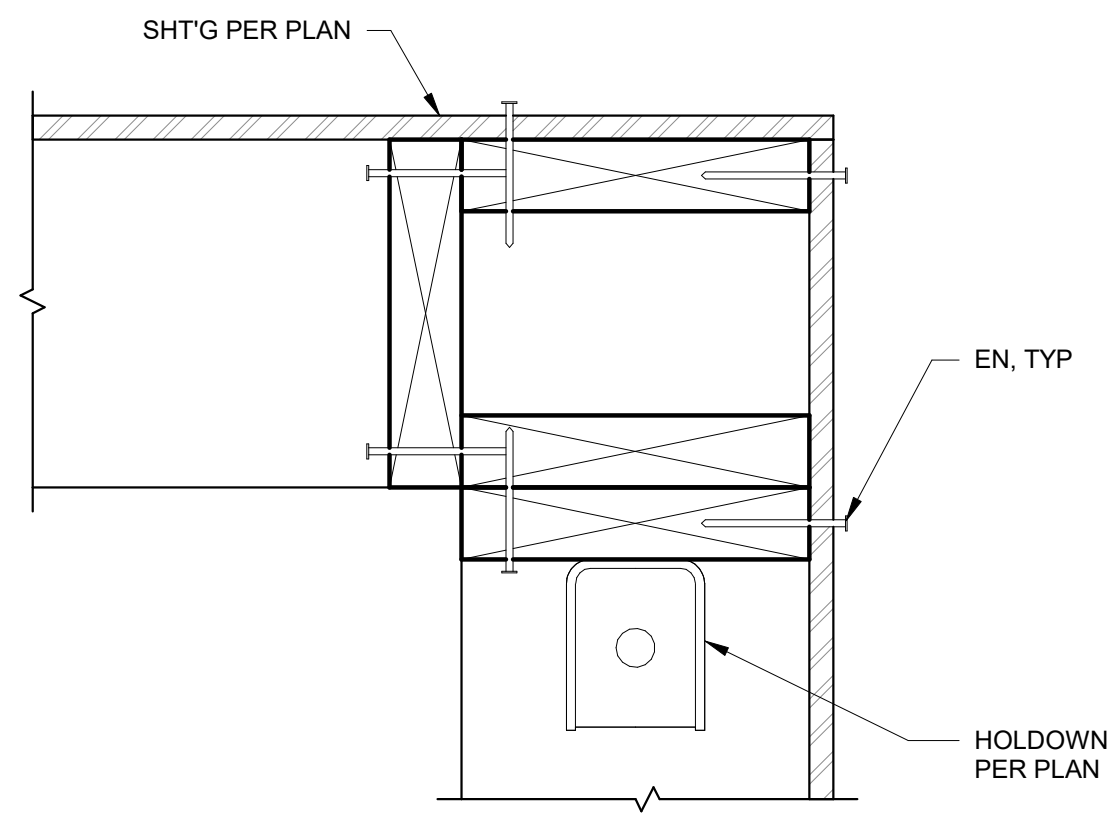
**3** TYPICAL WALL SECTION  
S1.2 SCALE: 3/4" = 1'-0"



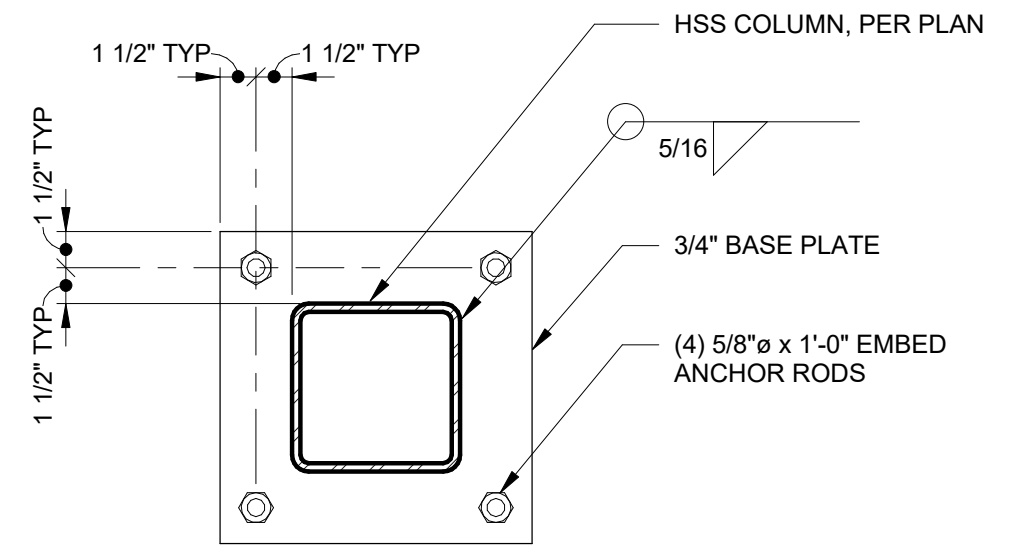
**4** TYPICAL POST-BEAM CONNECTION DETAIL  
S1.2 SCALE: 1 1/2" = 1'-0"

NOTES:  
1. USE CCQ AT MID BEAM CONNECTIONS. USE ECCQ AT EDGE BEAM CONNECTIONS. EXAMPLE SHOWS ECCQ.

NOTE: FOR ADDITIONAL INFORMATION SEE 2 / S1.2

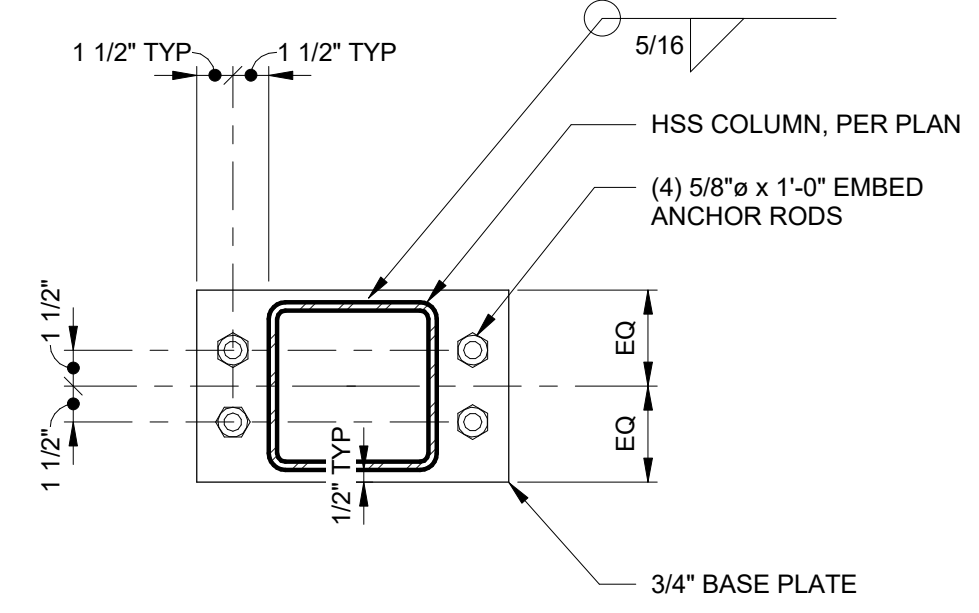


**5** TYPICAL PLAN @ CORNER  
S1.2 SCALE: 3" = 1'-0"



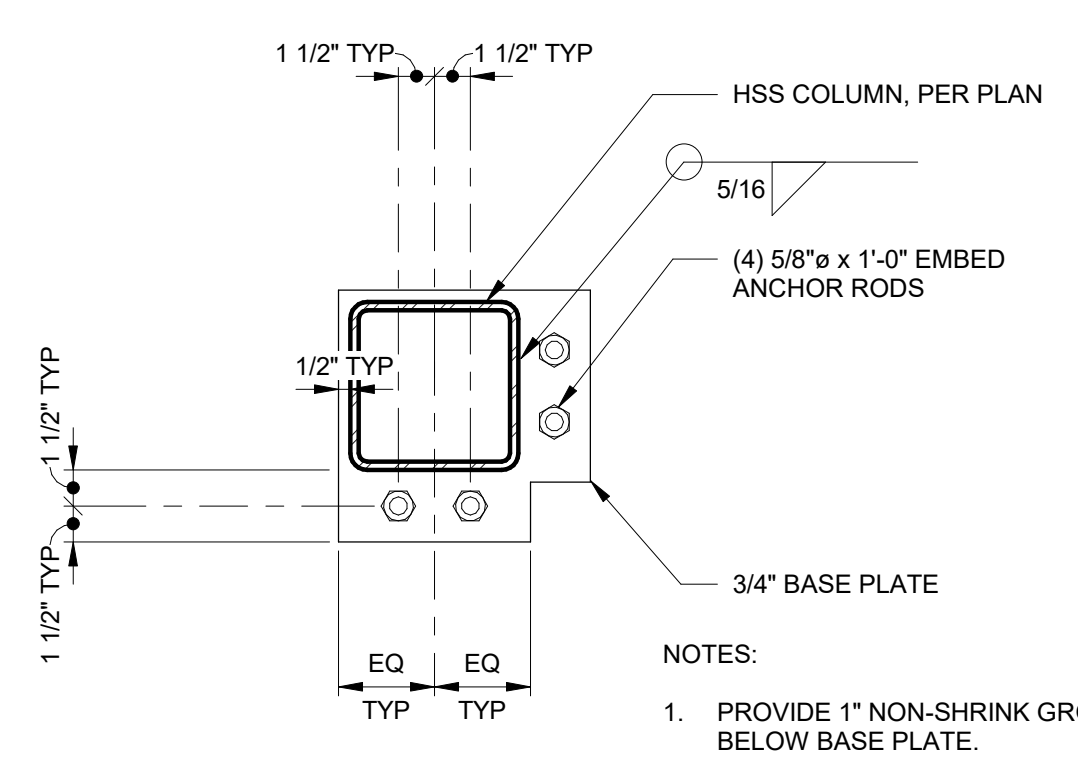
**6** TYPICAL HSS BASE PLATE  
S1.2 SCALE: 1 1/2" = 1'-0"

NOTES:  
1. PROVIDE 1" NON-SHRINK GROUT BELOW BASE PLATE.



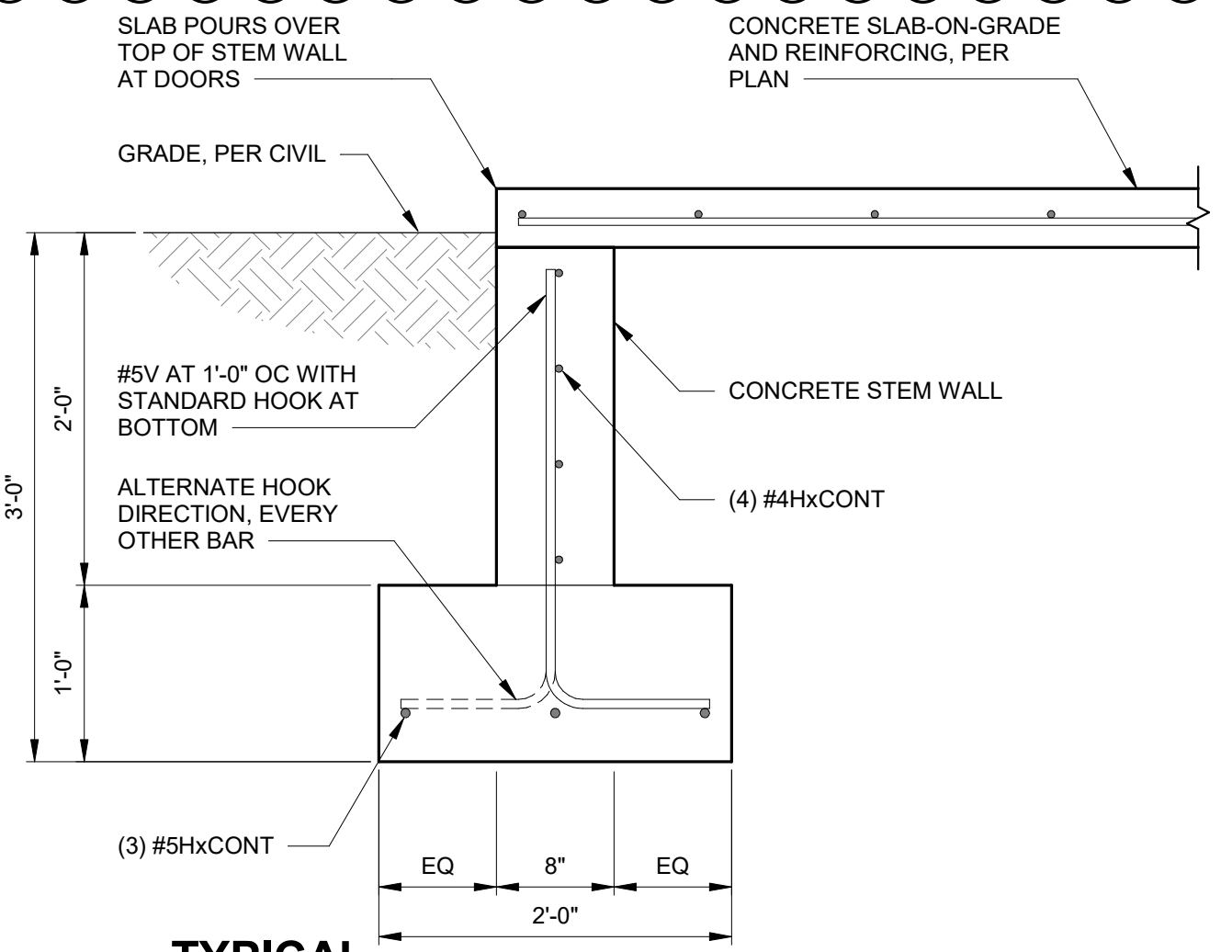
**7** TYPICAL HSS BASE PLATE AT STEM WALL  
S1.2 SCALE: 1 1/2" = 1'-0"

NOTES:  
1. PROVIDE 1" NON-SHRINK GROUT BELOW BASE PLATE.

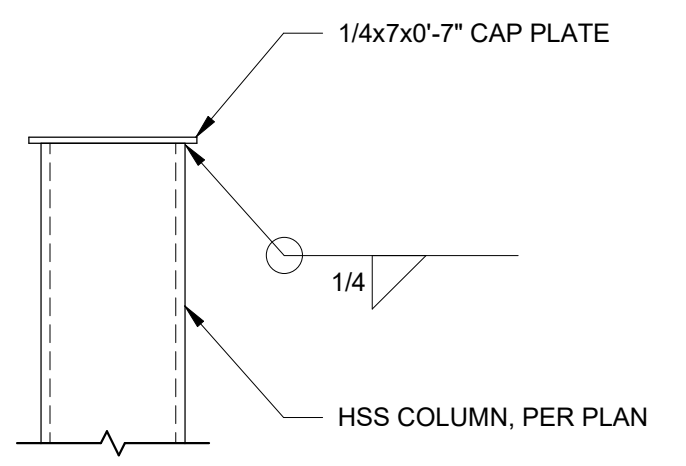


**8** TYPICAL HSS BASE PLATE AT CORNER  
S1.2 SCALE: 1 1/2" = 1'-0"

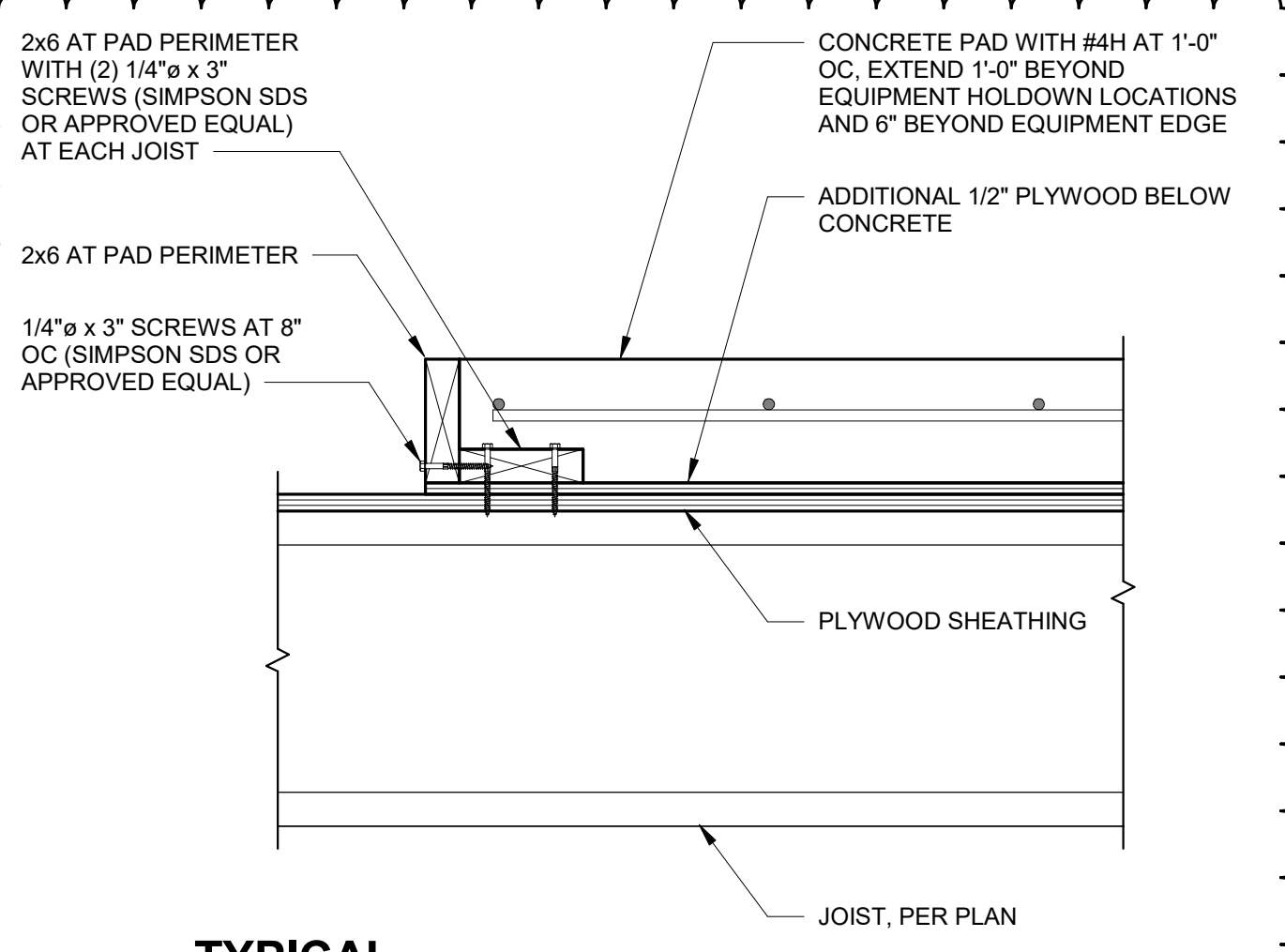
NOTES:  
1. PROVIDE 1" NON-SHRINK GROUT BELOW BASE PLATE.



**9** TYPICAL EXTERIOR STEM WALL SECTION AT DOOR  
S1.2 SCALE: 1" = 1'-0"



**10** TYPICAL HSS COLUMN CAP  
S1.2 SCALE: 1 1/2" = 1'-0"



**11** TYPICAL SECOND FLOOR HOUSEKEEPING PAD  
S1.2 SCALE: 1 1/2" = 1'-0"

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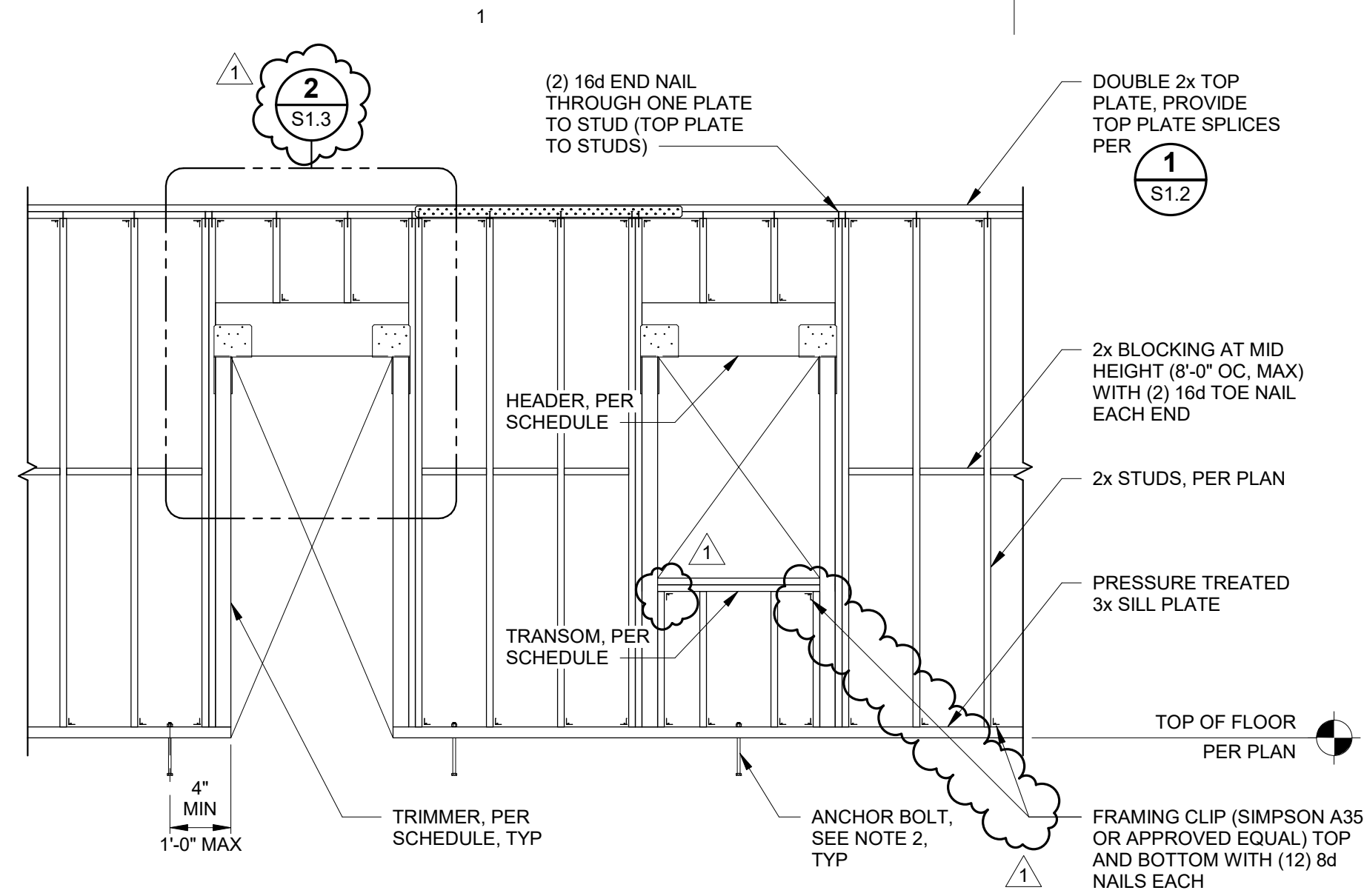
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No.	Description	Date
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Sheet Contents  
TYPICAL FRAMING DETAILS

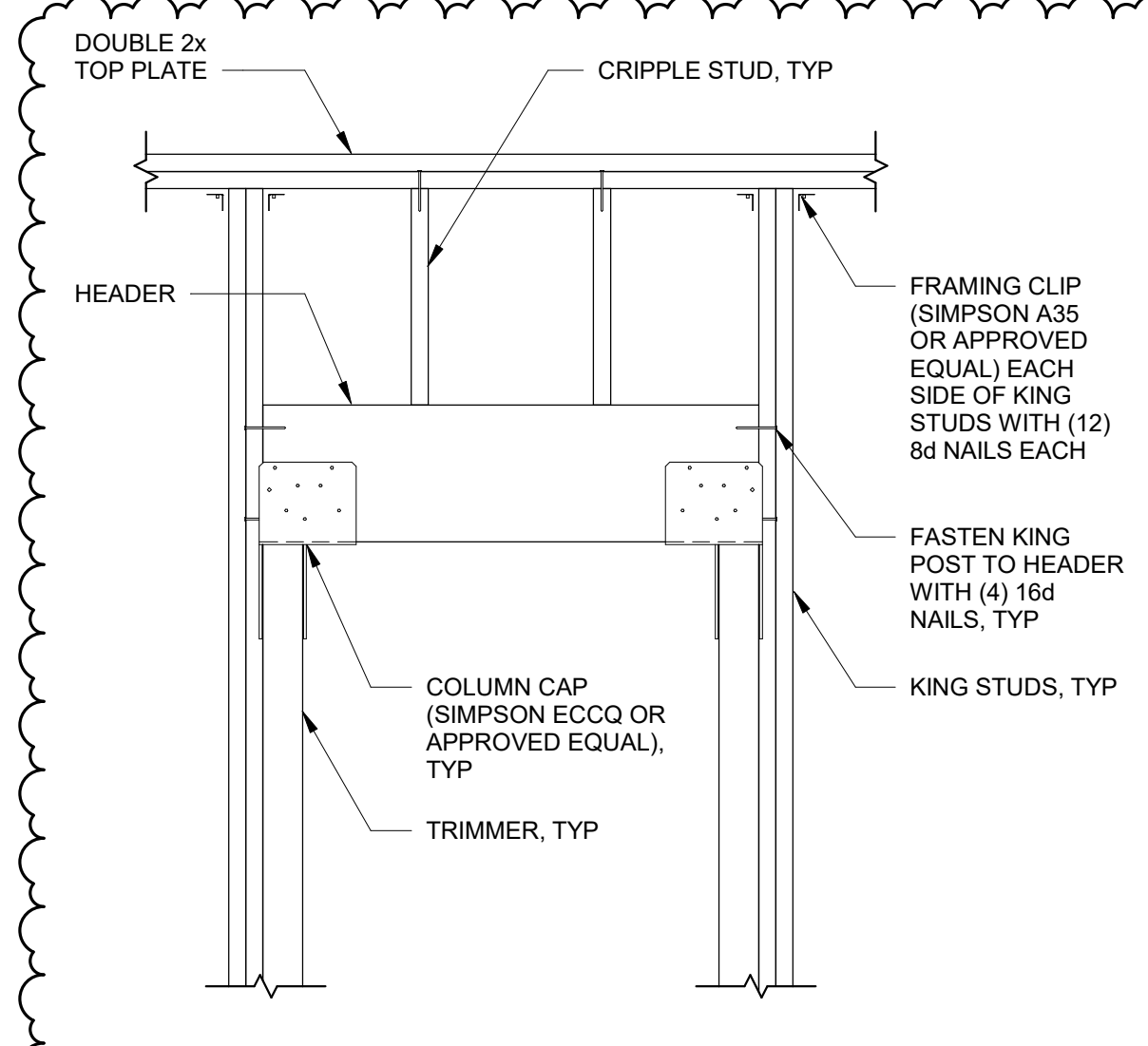
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**S1.2**

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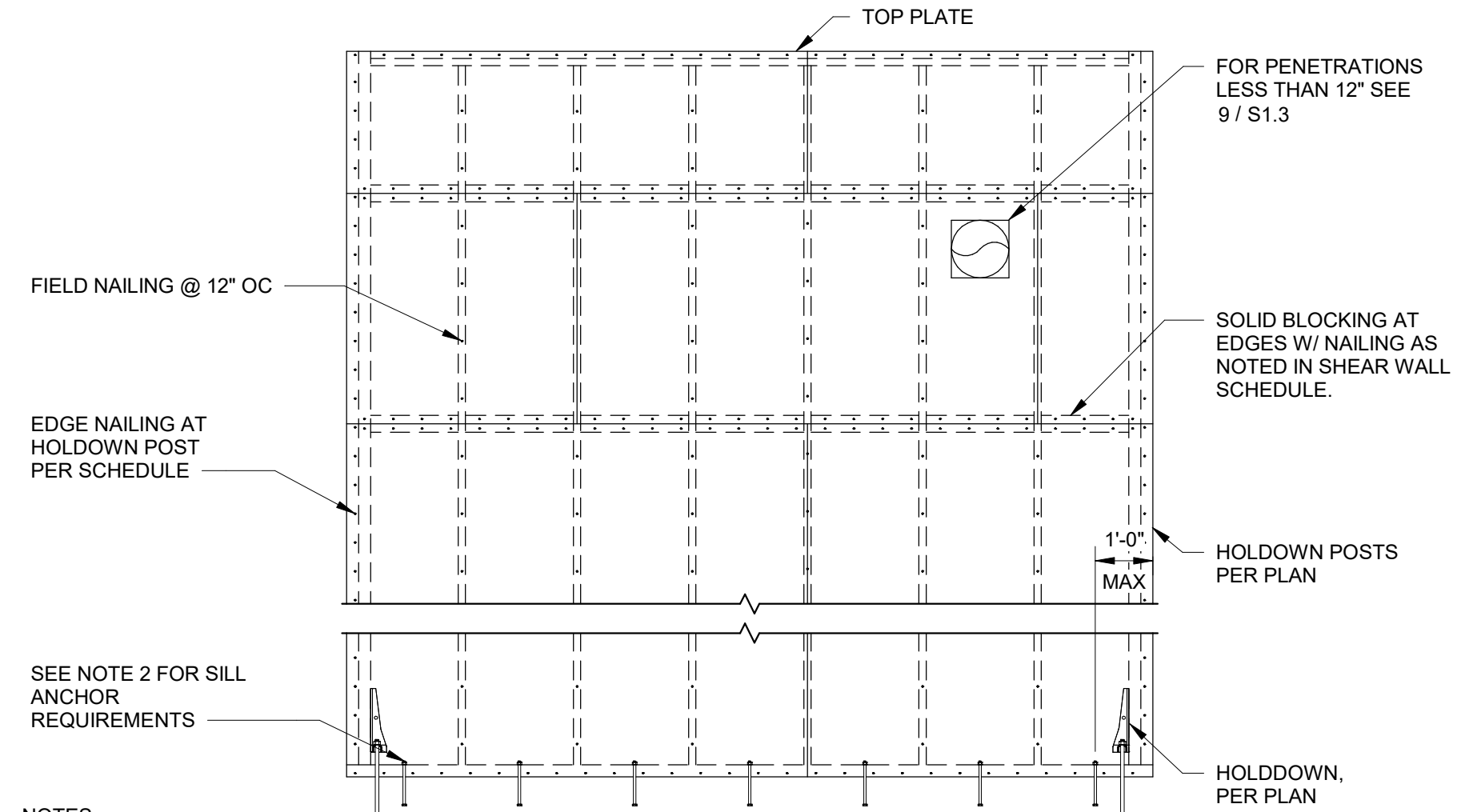
**1** TYPICAL BEARING WALL FRAMING DETAIL  
S1.3 SCALE: 3/8" = 1'-0"

- NOTES:
- HEADERS, KING STUDS AND OTHER REFERENCES ON PLAN GOVERN OVER THE TYPICAL DETAIL.
  - MINIMUM SILL PLATE ANCHORAGE:
    - AT CONCRETE:
      - PROVIDE 5/8"ø x 7" MINIMUM EMBED ANCHOR BOLTS AT 2'-8" OC.
      - PROVIDE A MINIMUM OF (2) ANCHOR BOLTS PER WALL PIER.
    - AT ELEVATED WOOD DECK:
      - PROVIDE (2) 16d AT 1'-4" OC OR 1/4"ø x 3 1/2" LAG SCREWS (SIMPSON SDS OR APPROVED EQUAL) AT 2'-0" OC.
    - SEE SHEAR WALL SCHEDULE FOR ADDITIONAL ANCHORAGE AT SHEAR WALLS.
  - FASTEN MULTI-PLY KING POSTS TOGETHER WITH (2) 16d NAILS AT 1'-0" OC, MIN.



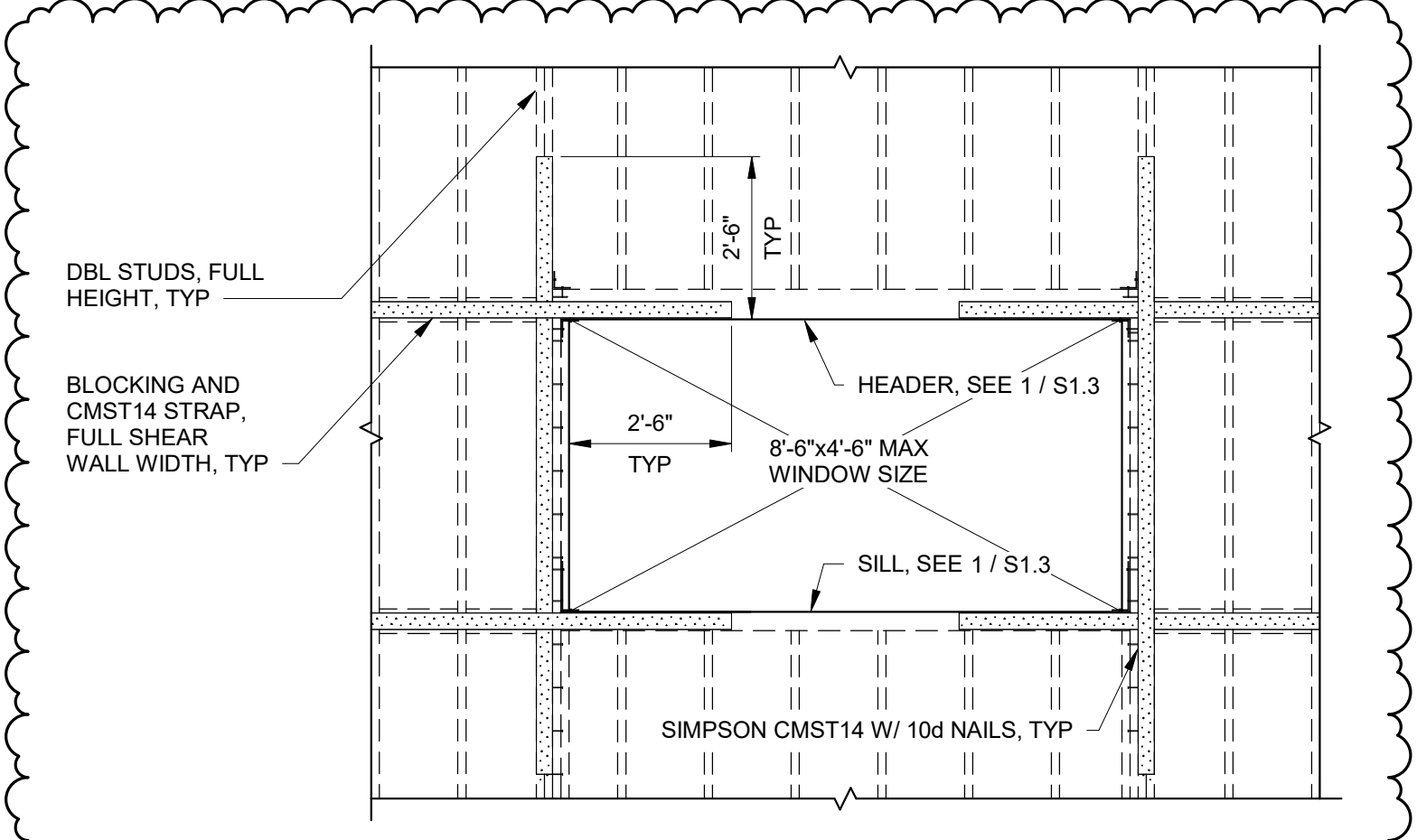
**2** TYPICAL HEADER DETAIL  
S1.3 SCALE: 3/4" = 1'-0"

HEADER SCHEDULE					
SPAN	HEADER	TRIMMERS	POST CAP	KING STUDS	TRANSOM
4'-0" AND LESS	GLB 6 3/4x9	4x8	SIMPSON ECCQ 74SDS2.5 OR APPROVED EQUAL	(2) 2x8	(2) 2x8
4'-0" TO 8'-0"	GLB 6 3/4x14	8x8	SIMPSON ECCQ 76SDS2.5 OR APPROVED EQUAL	(3) 2x8	(2) 2x8



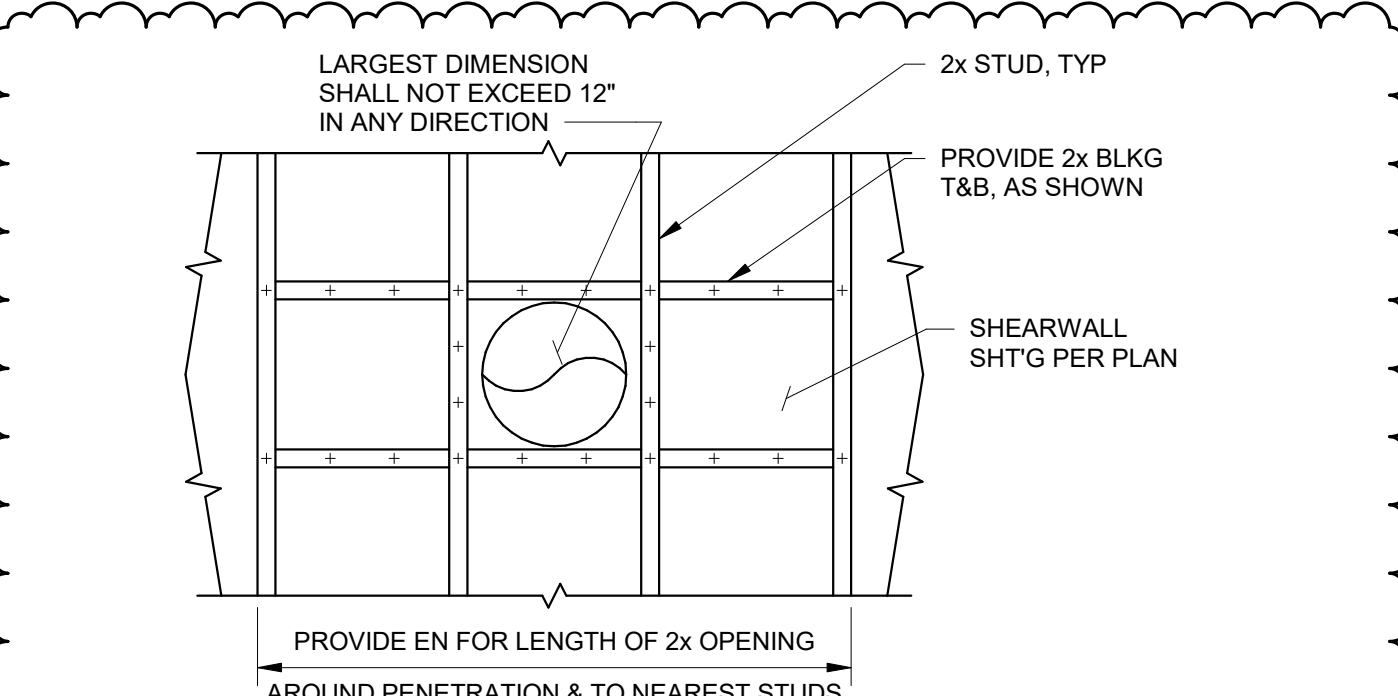
- NOTES:
- ALL FASTENERS TO BE 10d COMMON NAILS. SUBMIT LITERATURE FOR ALTERNATIVE FASTENERS FOR APPROVAL AS PER GENERAL NOTES.
  - SILL NAILS OR ANCHOR BOLTS PER SCHEDULE W/ 3"x3"x2.229" PLATE WASHERS, COUNTER-SINK SHEAR WALL POST TO LOCATE FIRST ANCHOR BOLT WHEN POST AND HOLDDOWN CONFIGURATION DOES NOT ALLOW 1'-0" MAX DIMENSION.
  - PROVIDE 1/8" GAP BETWEEN EDGE AND SIDES OF SHEATHING PANELS.

**5** TYPICAL WOOD SHEAR WALL  
S1.3 SCALE: 3/8" = 1'-0"



- NOTES:
- INSTALL STRAPS OVER PLYWOOD SHEATHING.
  - INSTALL STRAPS ON BOTH SIDES @ DOUBLE SIDED SHEAR WALLS.

**6** TYPICAL WINDOW OPENINGS IN SHEAR WALLS  
S1.3 SCALE: 3/8" = 1'-0"



- NOTES:
- UNLESS INDICATED ON PLANS, COORDINATION OF MULTIPLE PENETRATIONS (2 OR MORE) SHALL BE APPROVED BY THE EOR.

**9** TYPICAL SHEAR WALL PENETRATION  
S1.3 SCALE: 3/4" = 1'-0"

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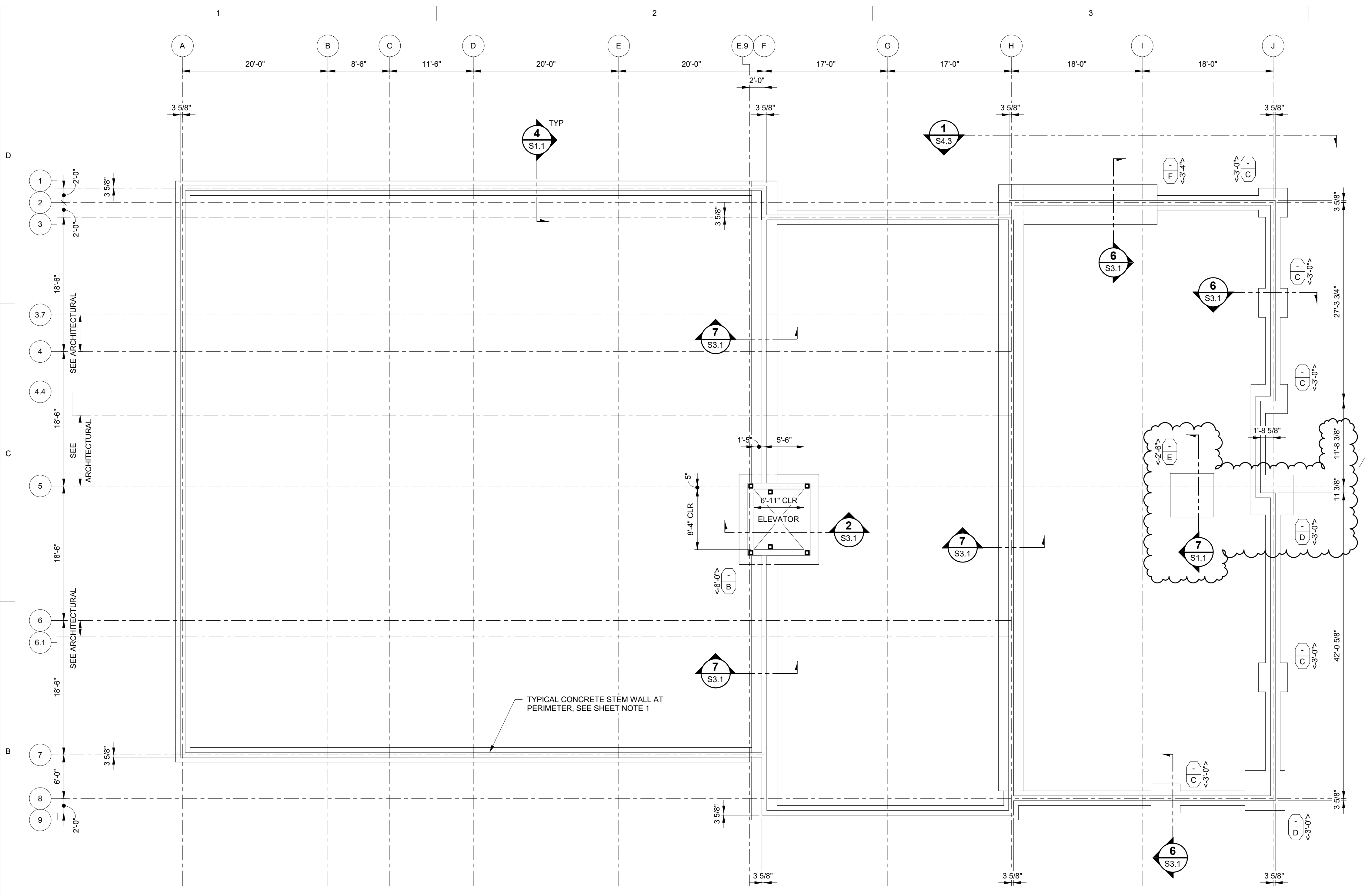
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YCHC ALTERNATE DESIGN  
FOUNDATION PACKAGE  
YAKUTAT, ALASKA

No.	Description	Date
1	REVISIONS	06/18/19

Drawn by DJM	Date 04/29/2019
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Sheet Contents  
TYPICAL FRAMING DETAILS

Sheet No.  
**S1.3**



**SHEET NOTES**

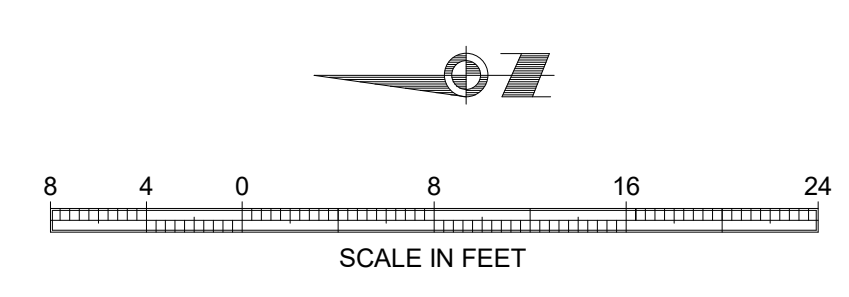
1. BOTTOM OF STEM WALL FOOTING SHALL BE 3'-0" BELOW GRADE, UNLESS NOTED OTHERWISE.

**LEGEND**

1. TYPICAL FOUNDATION TAG:
  - PILASTER OR PEDESTAL TYPE  
SEE PILASTER / PEDESTAL SCHEDULE
  - BOTTOM OF FOOTING ELEVATION  
RELATIVE TO GRADE LEVEL
  - FOOTING TYPE  
SEE FOOTING SCHEDULE

**1 FOUNDATION PLAN**  
S2.1 SCALE: 1/8" = 1'-0"

TYPE	DIMENSIONS			REINFORCEMENT							
	LENGTH (L)	WIDTH (W)	THICKNESS (T)	BOTTOM				TOP			
				QTY	SIZE	QTY	SIZE	QTY	SIZE	QTY	SIZE
A	6'-0"	6'-0"	1'-0"	(7)	#6	(7)	#6	(7)	#6	(7)	#6
B	12'-5"	11'-0"	2'-0"	(12)	#6	(13)	#6	(12)	#6	(13)	#6
C	4'-0"	4'-0"	1'-0"	(5)	#6	(5)	#6	-	-	-	-
D	5'-6"	5'-6"	1'-0"	(6)	#6	(6)	#6	-	-	-	-
E	6'-0"	6'-0"	1'-6"	(7)	#6	(7)	#6	-	-	-	-
F	21'-10"	5'-6"	1'-4"	(5)	#6	1'-0" OC	#6	(5)	#6	1'-0" OC	#6



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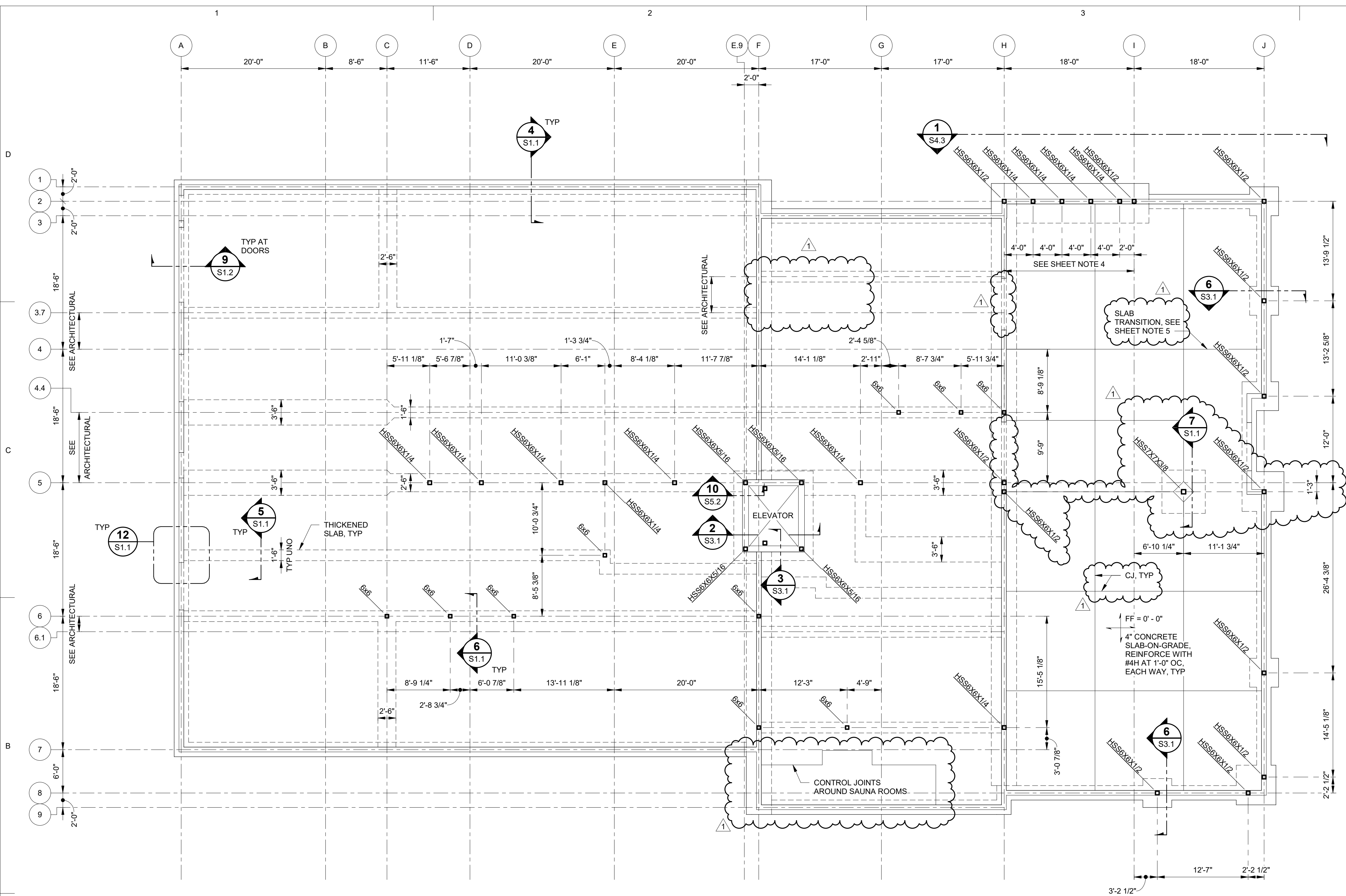
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Sheet Contents  
FOUNDATION PLAN

Sheet No.  
**S2.1**

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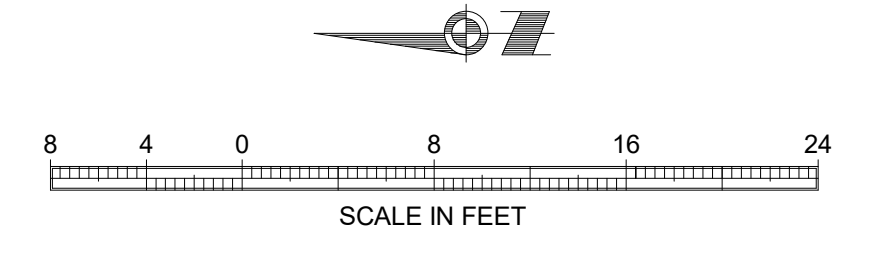
**1** FIRST FLOOR SLAB PLAN  
 S2.2 SCALE: 1/8" = 1'-0"

**SHEET NOTES**

- THE REFERENCE ELEVATION OF THE FIRST FLOOR IS 0'-0". THE TOP OF CONCRETE OF THE FIRST FLOOR SLAB-ON-GRADE IS AT THE REFERENCE ELEVATION, UNLESS NOTED OTHERWISE.
- LOCATE CENTER-LINE OF THICKENED SLABS BELOW CENTER-LINE OF WALLS. SEE ARCHITECTURAL FOR WALL LOCATIONS.
- CONTROL JOINTS:
  - NORTH OF GRID H:
    - CONTROL JOINTS SHALL BE LOCATED AT GRID LINES. FOR GRID LINES FALLING ABOVE A THICKENED SLAB, LOCATE CONTROL JOINTS AT A SHALLOW EDGE OF THE THICKENED SLAB.
  - SOUTH OF GRID H AND CLINIC AREA:
    - AT COVERED FLOORS, LOCATE CONTROL JOINTS AT 16'-0" OC, MAX.
    - AT EXPOSED CONCRETE FLOORS, LOCATE CONTROL JOINTS PER ARCHITECTURAL.
- HSS6x6 COLUMN BELOW EACH GLU-LAM ROOF BEAM.
- SEE ARCHITECTURAL FOR AREAS OF RAISED/EXPOSED CONCRETE SLAB.

**LEGEND**

- TYPICAL COLUMN TAG:
  - MEMBER SIZE AS NOTED ON PLANS
  - COLUMN SYMBOLIC REPRESENTATION



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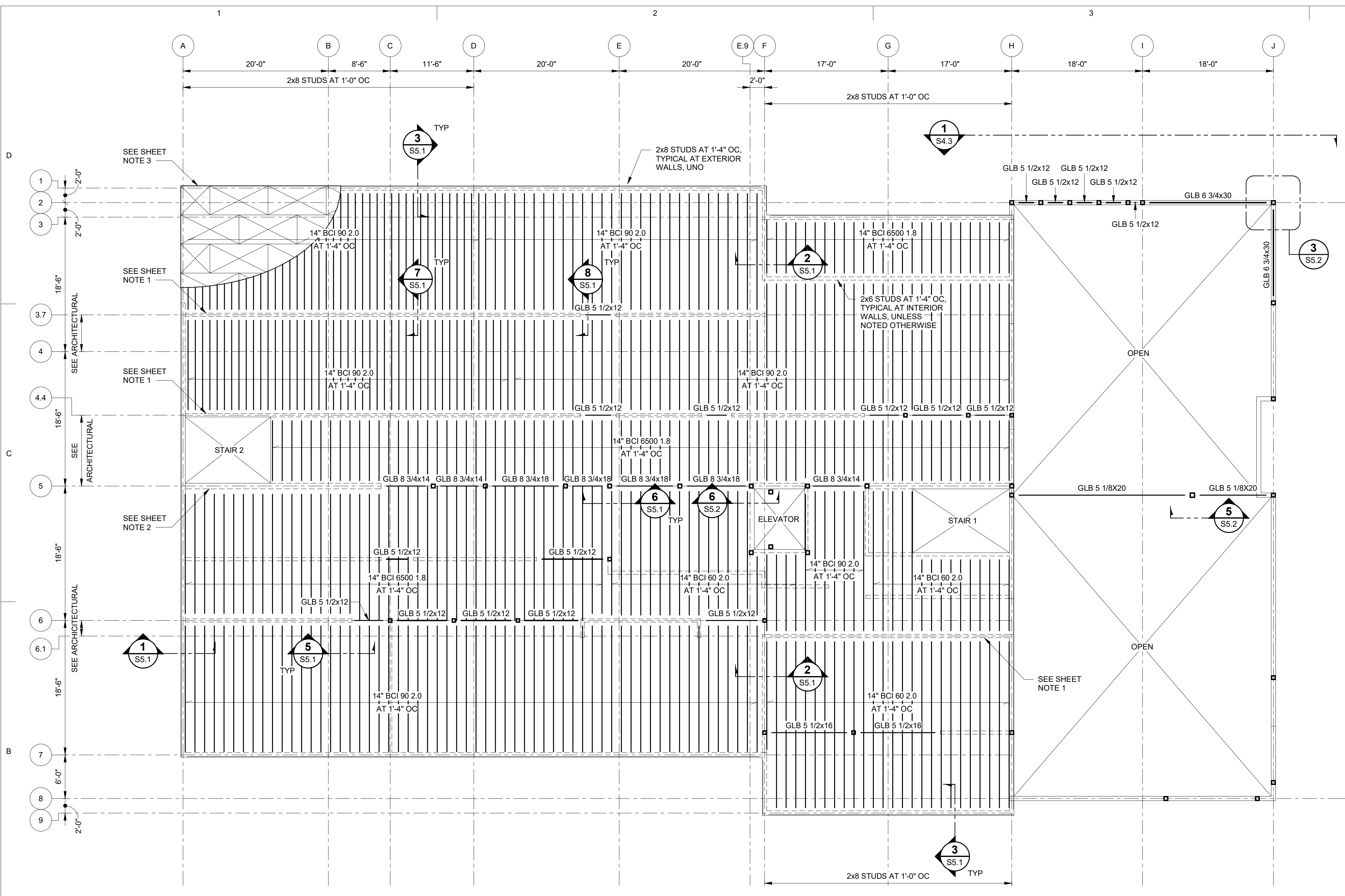
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Sheet Contents  
 FIRST FLOOR SLAB PLAN

Sheet No.  
**S2.2**





**1 SECOND FLOOR FRAMING PLAN**  
 S2.3 SCALE: 1/8" = 1'-0"

**SHEET NOTES**

- STUDS SHALL BE PLACED AT 1'-0" OC FROM GRID LINES A TO D AT GRID LINES 3.7 AND 4.4.
- STUDS SHALL BE 2x10 AT 1'-4" OC AT GRID LINE 5.
- 1 1/8" PLYWOOD:
  - A. 8d at 6" OC EDGE AND BOUNDARY NAILING.
  - B. 8d at 1'-0" OC FIELD.
  - C. BOUNDARY NAILING SHALL OCCUR AT EXTERIOR WALLS AND ALONG GRIDS C, E.9, F, H, 4.4 AND 5.0.

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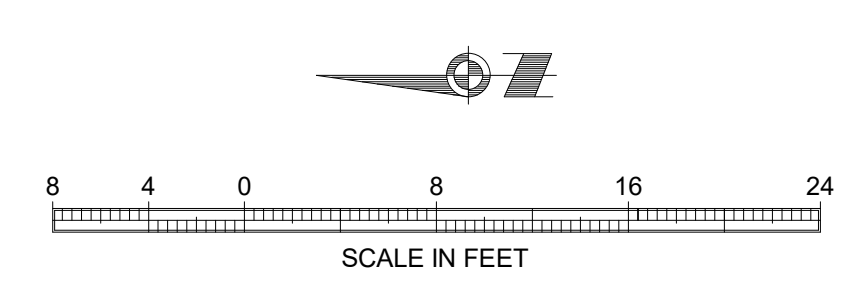
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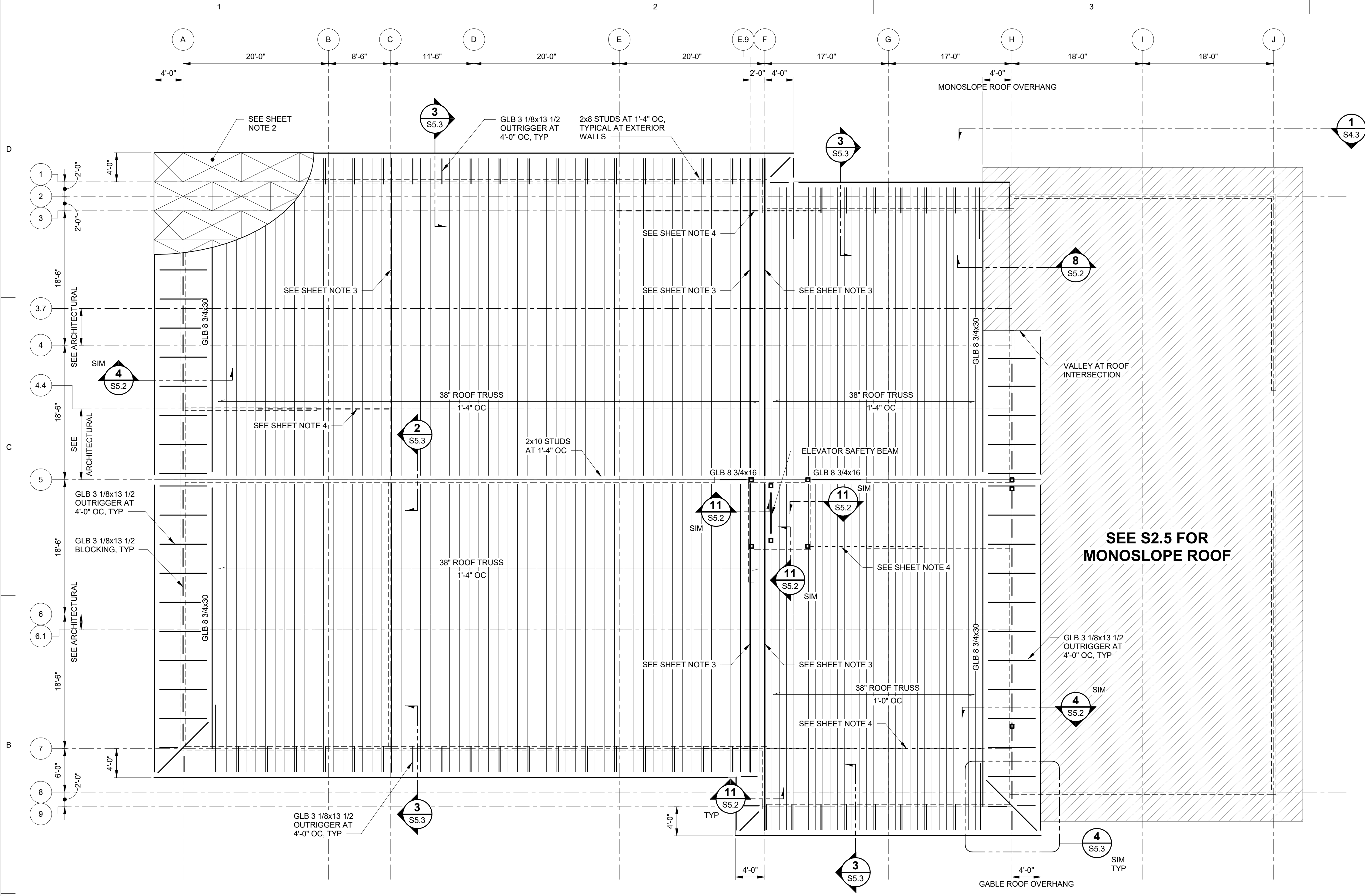
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Sheet Contents  
 SECOND FLOOR FRAMING PLAN

Sheet No.  
**S2.3**





**1 GABLE ROOF FRAMING PLAN**  
 S2.4 SCALE: 1/8" = 1'-0"

**SHEET NOTES**

- ROOF TRUSSES SHALL BE REDBUILT RED-M OR APPROVED EQUAL.
- 3/4" PLYWOOD WITH 4'-0" SPAN RATING:
  - A. 8d AT 6" OC EDGE AND 4" OC BOUNDARY NAILING.
  - B. 8d AT 1'-0" OC FIELD.
  - C. BOUNDARY NAILING SHALL OCCUR ABOVE ALL EXTERIOR WALL BLOCKING AND ALONG GRIDS E.9, F AND H.
- TRUSS SHALL ACT AS A COLLECTOR. DESIGN TRUSS FOR 400 LB/FT SEISMIC FORCE.
- BLOCK AND STRAP (SIMPSON CMST14 OR APPROVED EQUAL), CONTINUE STRAP 8'-0" ACROSS TOP OF SHEAR WALL:
  - A. FROM GRIDS E TO F ON GRID 3
  - B. FROM GRIDS F TO H ON GRID 7
  - C. FROM GRIDS A TO C ON GRID 4.4
  - D. FROM GRIDS G.9 TO H ON GRID 5

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 SE 13950

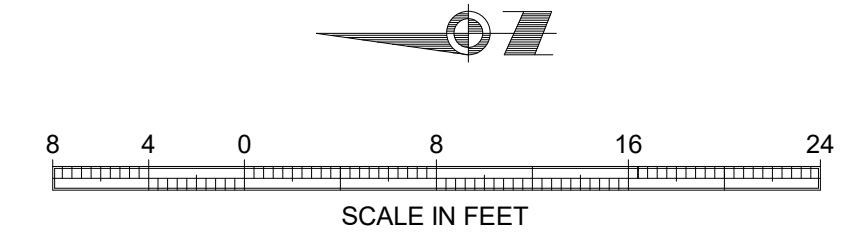
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 YAKUTAT, ALASKA

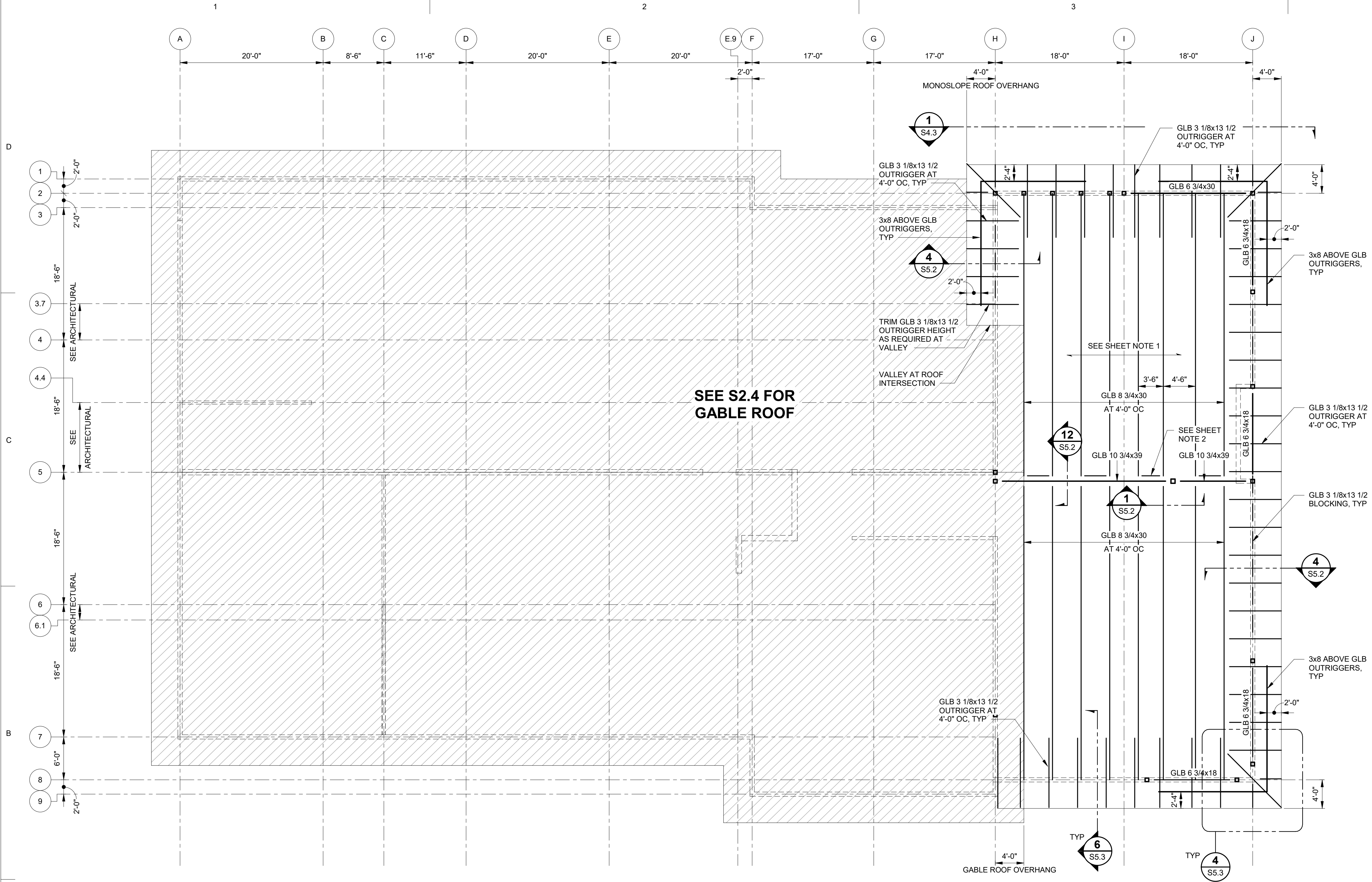
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No.	Description	Date

Drawn by DBH / DJM	Date 06/18/2019
Checked AKM	Job No. 17341JN

Sheet Contents  
 GABLE ROOF FRAMING PLAN

Sheet No.  
**S2.4**





**1 MONOSLOPE ROOF FRAMING PLAN**  
 S2.5 SCALE: 1/8" = 1'-0"

**SHEET NOTES**

1. 5/8" PLYWOOD:
  - A. 8d at 6" OC EDGE AND BOUNDARY NAILING.
  - B. 8d at 1'-0" OC FIELD NAILING.
  - C. OVER 3x6 TONGUE AND GROOVE DECKING:
    - a. CONNECT TONGUE AND GROOVE DECKING TO GLULAM BEAMS WITH (3) 16d NAILS IN EACH MEMBER.
2. PROVIDE BLOCKING IN EVERY OTHER BAY BETWEEN 8 3/4 GLBS, ABOVE 10 3/4" GLB.

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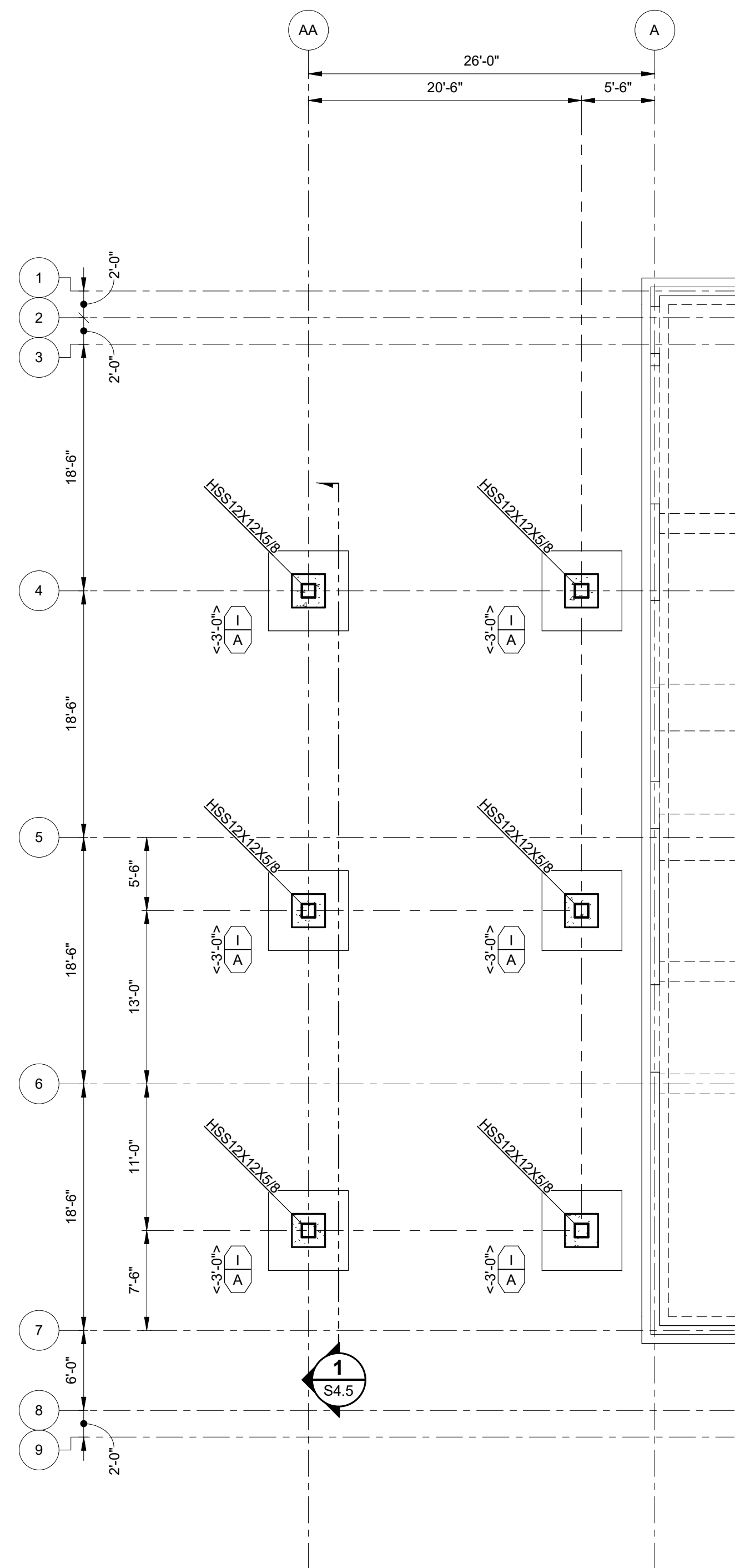
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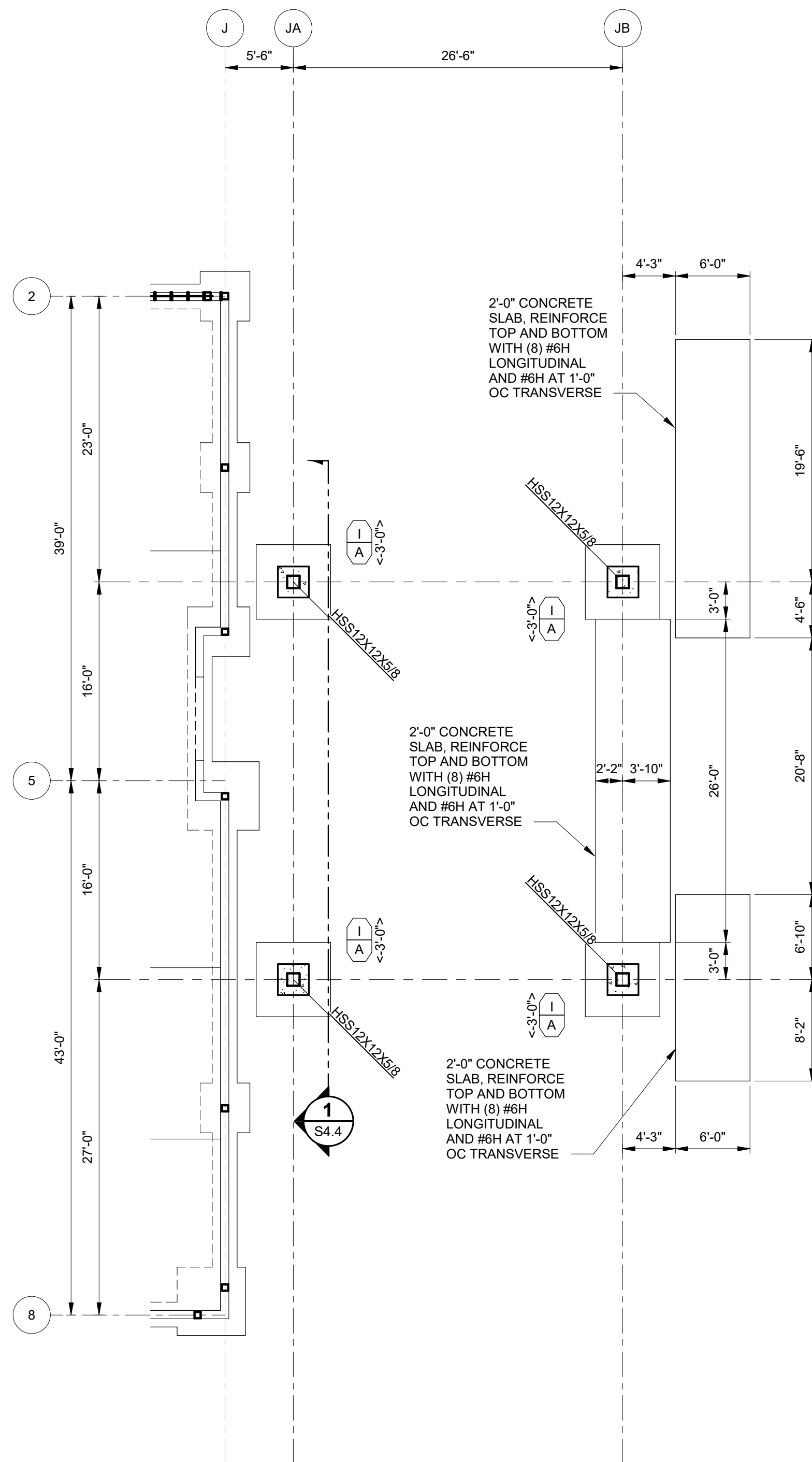
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Checked AKM	Job No. 17341JN

Sheet Contents MONOSLOPE ROOF FRAMING PLAN
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Sheet No.  
**S2.5**



**1 NORTH CANOPY FOUNDATION PLAN**  
S2.6 SCALE: 1/8" = 1'-0"

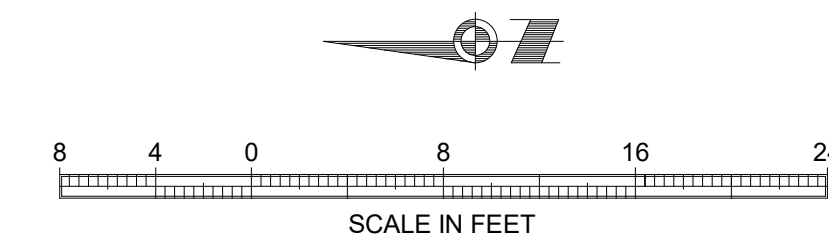


**2 SOUTH CANOPY FOUNDATION PLAN**  
S2.6 SCALE: 1/8" = 1'-0"

- LEGEND**
- TYPICAL FOUNDATION TAG:
    - PILASTER OR PEDESTAL TYPE  
SEE PILASTER / PEDESTAL SCHEDULE
    - BOTTOM OF FOOTING ELEVATION  
RELATIVE TO GRADE LEVEL
    - FOOTING TYPE  
SEE FOOTING SCHEDULE
  - TYPICAL COLUMN TAG:
    - MEMBER SIZE AS NOTED ON  
PLANS
    - COLUMN SYMBOLIC  
REPRESENTATION

FOOTING SCHEDULE											
TYPE	DIMENSIONS			REINFORCEMENT							
	LENGTH (L)	WIDTH (W)	THICKNESS (T)	BOTTOM				TOP			
				QTY	SIZE	QTY	SIZE	QTY	SIZE	QTY	SIZE
A	6'-0"	6'-0"	1'-0"	(7)	#6	(7)	#6	(7)	#6	(7)	#6
B	12'-5"	11'-0"	2'-0"	(12)	#6	(13)	#6	(12)	#6	(13)	#6
C	4'-0"	4'-0"	1'-0"	(5)	#6	(5)	#6	-	-	-	-
D	5'-6"	5'-6"	1'-0"	(6)	#6	(6)	#6	-	-	-	-
E	6'-0"	6'-0"	1'-6"	(7)	#6	(7)	#6	-	-	-	-
F	21'-10"	5'-6"	1'-4"	(5)	#6	1'-0" OC	#6	(5)	#6	1'-0" OC	#6

PEDESTAL SCHEDULE							
TYPE	DIMENSIONS		TIES		LONGITUDINAL		REMARKS
	WIDTH	DEPTH	QTY	SIZE	QTY	SIZE	
I	2'-6"	2'-6"	AT 6" OC	#4	(8)	#8	SEE DETAIL 1/S3.1



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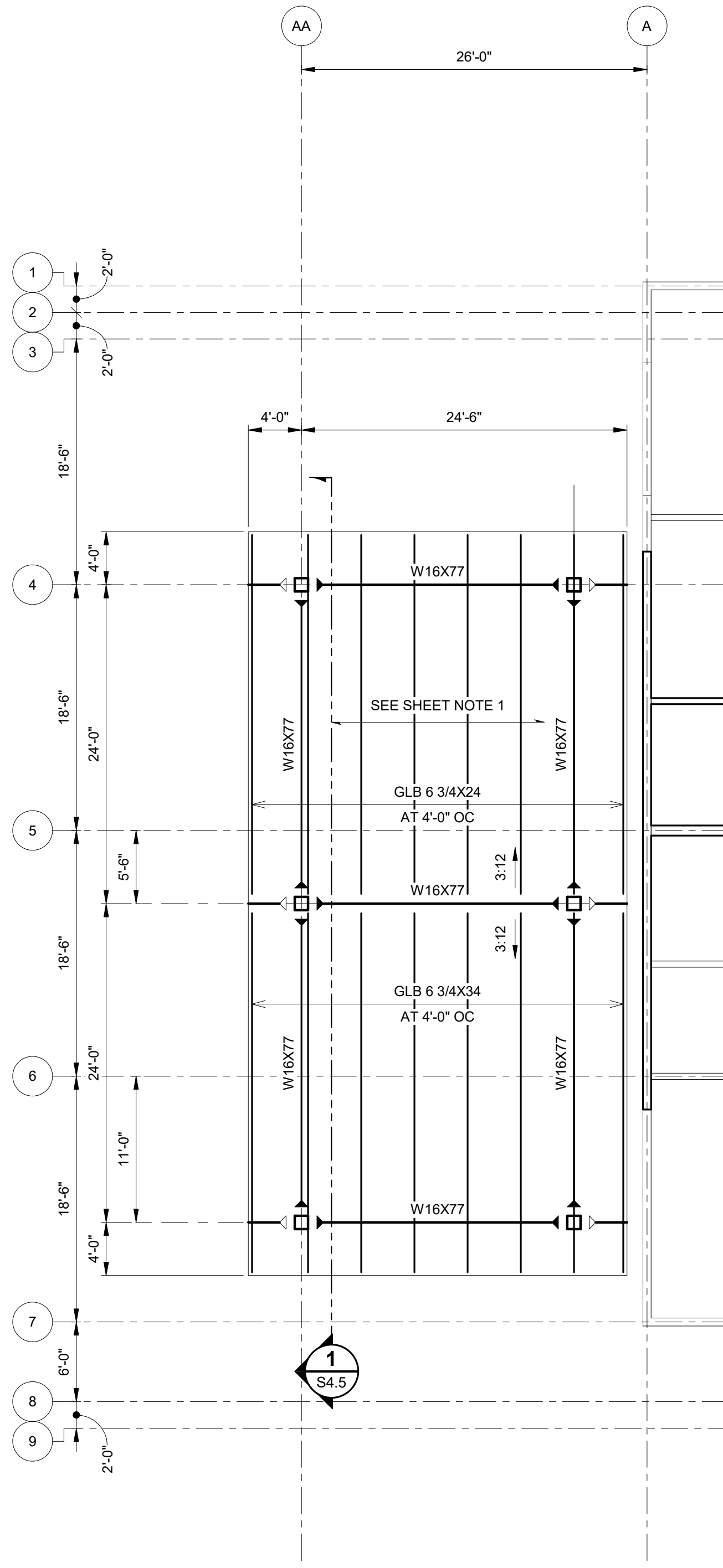
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YCHC ALTERNATE DESIGN  
FOUNDATION PACKAGE  
YAKUTAT, ALASKA

Revisions		
No.	Description	Date

Drawn by DJM	Date 06/18/2019
Checked AKM	Job No. 17341JN

Sheet Contents  
CANOPY FOUNDATION PLANS

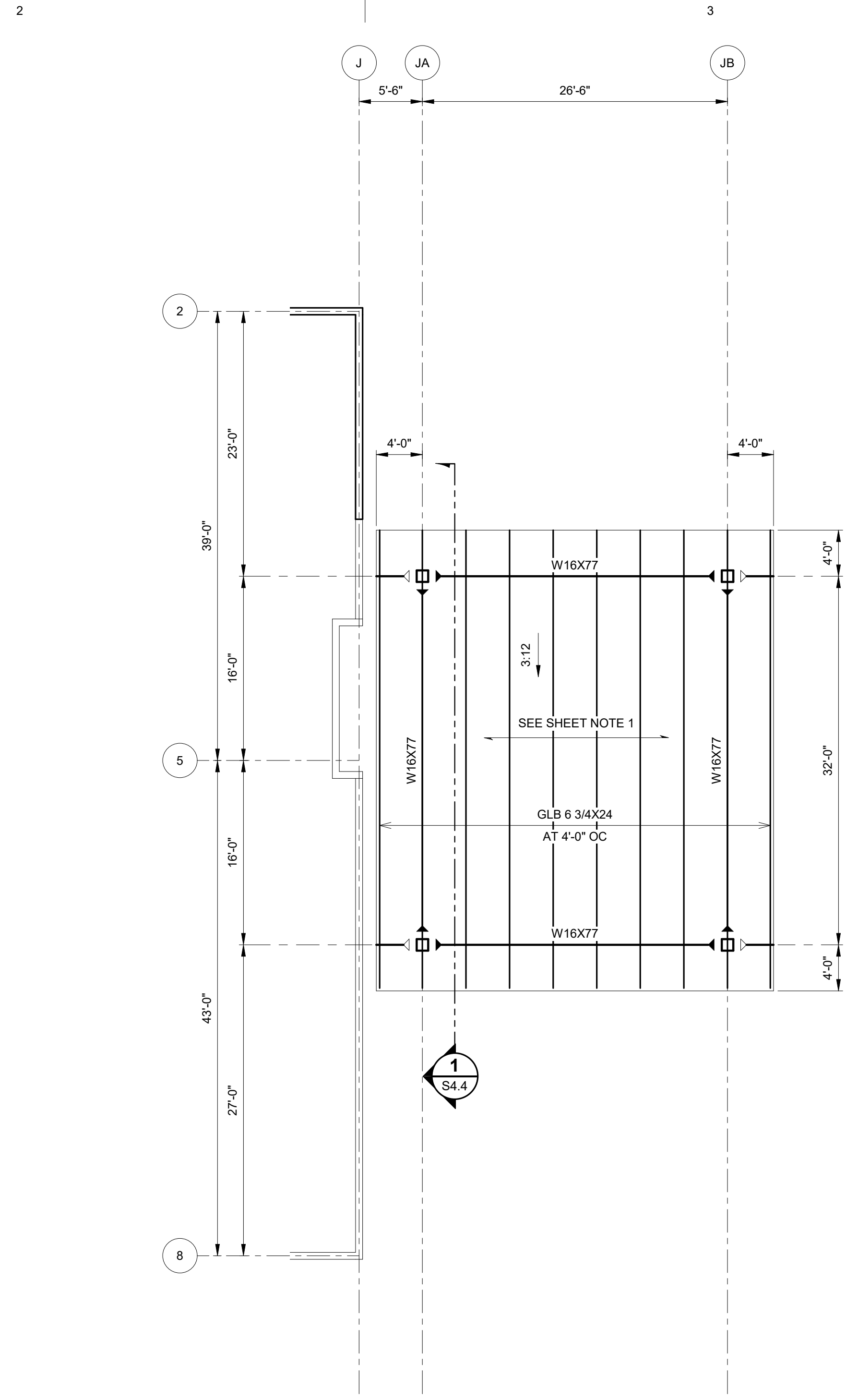
Sheet No.  
**S2.6**



1

**NORTH CANOPY ROOF FRAMING PLAN**

S2.7 SCALE: 1/8" = 1'-0"



2

**SOUTH CANOPY ROOF FRAMING PLAN**

S2.7 SCALE: 1/8" = 1'-0"

**SHEET NOTES**

1. 5/8" PLYWOOD:
  - A. 10d AT 6" OC EDGE NAILING.
  - B. 10d AT 1'-0" OC FIELD NAILING.
  - C. OVER 3x6 TONGUE AND GROOVE DECKING:
    - a. CONNECT TONGUE AND GROOVE DECKING TO GLULAM BEAMS WITH (3) 16d NAILS IN EACH MEMBER.

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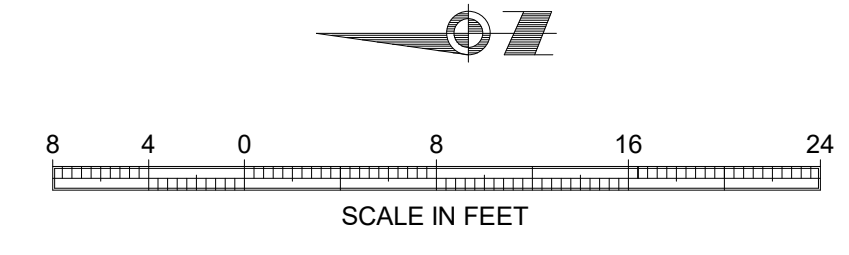
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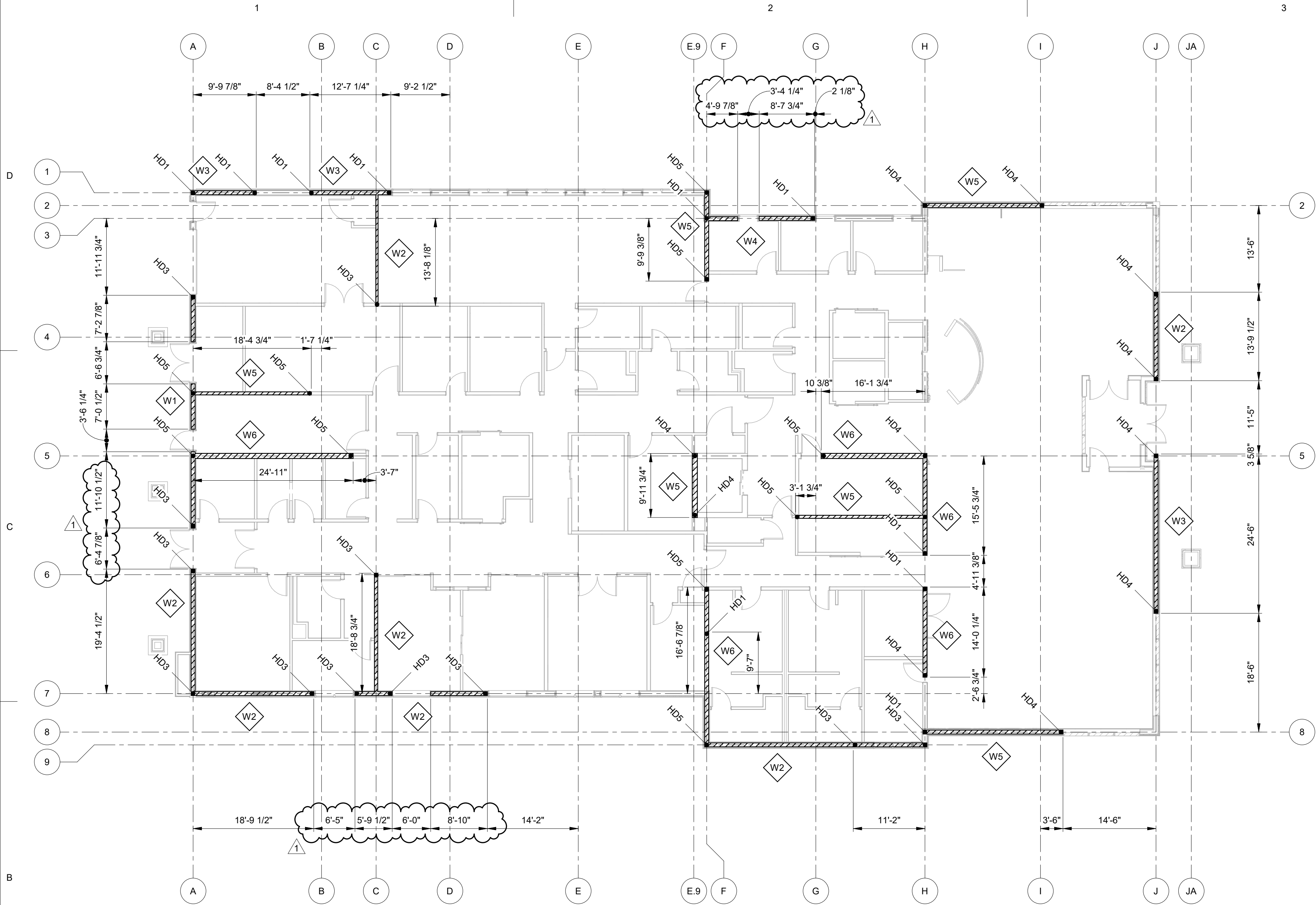
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No.	Description	Date

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Sheet Contents  
 CANOPY ROOF FRAMING PLANS

Sheet No.  
**S2.7**





**1** FIRST FLOOR SHEAR WALL PLAN  
S2.8 SCALE: 3/32" = 1'-0"

HOLDOWN SCHEDULE					
MARK	SIMPSON HOLDOWN (OR APPROVED EQUAL)	MINIMUM CHORD	FASTENERS AT CHORD	ANCHOR BOLT	NOTES
HD1	HD12	6x6	(4) 1"Ø BOLTS	1 1/8"Ø	-
HD2	HD19	6x6	(5) 1"Ø BOLTS	1 1/4"Ø	-
HD3	HDU11-SDS2.5	8x8	(30) 1/4"Ø x 2 1/2" SDS	1"Ø	-
HD4	HSS	6x6	-	(4) 5/8"Ø	-
HD5	(2) HD12	6x6	(4) 1"Ø BOLTS AT EACH HD12	(2) 1 1/8"Ø	SEE DETAIL 8/S3.1
HD6	MSTC40	(2) 2x	(28) 0.148"Ø x 3 1/4" SD	-	STRAP
HD7	MSTC66	(2) 2x	(64) 0.148"Ø x 3 1/4" SD	-	STRAP

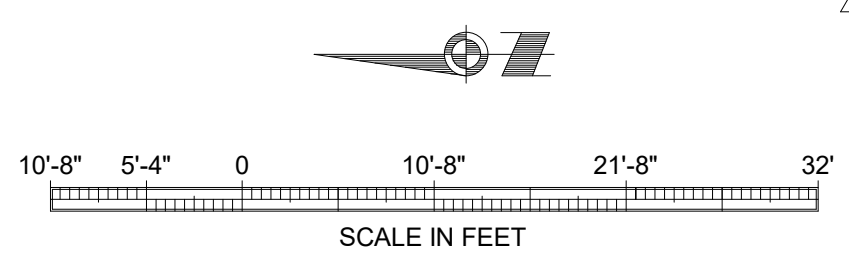
**HOLDOWN SCHEDULE NOTES**

1. PROVIDE PANEL EDGE NAILING PER SHEAR WALL SCHEDULE AT HOLDOWN STUDS AND AT PANEL EDGES.
2. HOLDOWNS LOCATED PER PLAN.
3. HOLDOWNS SHALL BE SIMPSON STRONG TIE OR APPROVED EQUAL.
4. SEE DETAILS FOR HOLDOWN CONFIGURATION.

**SHEAR WALL SCHEDULE NOTES**

1. INSTALL PANELS EITHER HORIZONTALLY OR VERTICALLY.
2. WHERE SHEATHING IS APPLIED ON BOTH SIDES OF WALL, PANEL EDGE JOINTS ON 2x FRAMING SHALL BE STAGGERED SO THAT JOINTS ON THE OPPOSITE SIDES ARE NOT LOCATED ON THE SAME STUD.
3. BLOCKING IS REQUIRED AT ALL PANEL EDGES.
4. PROVIDE SHEAR WALL SHEATHING AND NAILING FOR THE ENTIRE LENGTH OF THE WALLS INDICATED ON THE PLANS. SEE PLANS FOR HOLDOWN REQUIREMENTS.
5. SHEATHING EDGE NAILING IS REQUIRED AT ALL HOLDOWN POSTS.
6. INTERMEDIATE FRAMING MEMBERS SHALL BE 2x.
7. FRAMING CLIPS: SIMPSON STRONG TIE A35 OR LTP5 EQUIV.
8. PROVIDE DOUBLE JOIST, RIM OR EQUAL WHERE PLATE ATTACHMENT SPECIFIES (2) ROWS OF NAILS.
9. ANCHOR BOLTS SHALL BE PROVIDED WITH STEEL PLATE WASHERS 1/4"x3"x3".
10. PROVIDE HOT-DIPPED GALVANIZED NAILS AND CONNECTOR PLATES FOR ALL CONNECTORS IN CONTACT WITH PRESSURE TREATED FRAMING MEMBERS.
11. OSB SHEATHING CAN BE USED IF APPROVED BY ENGINEER OF RECORD.
12. (2) 2X STUDS NAILED TOGETHER MAY BE USED IN PLACE OF A SINGLE 3X STUD. (2) 2X STUDS SHALL BE JOINED TOGETHER BY USING PLATE NAILING REQUIREMENTS.

SHEAR WALL SCHEDULE								
WALL TYPE	WALL APA-RATED SHEATHING [1] [2] [4] [12]	NAIL SIZE AND SPACING AT EDGES [4] [5]	NAIL SIZE AND SPACING AT INTERMEDIATE FRAMING MEMBERS [6]	STUD AND BLOCKING SIZE AD ADJOINING EDGES [3] [4]	SILL PLATE SIZE [10]	ANCHOR BOLT TO CONCRETE BELOW [10]	SILL PLATE NAILING	RIM JOIST OR BLOCKING CONNECTION TO TOP PLATE [7] SPACING
W1	15/32" CD-EXT ON (1) SIDE	8d AT 6" OC	8d AT 1'-0" OC	2x	2x	5/8"Ø x 8" EMBED AT 4'-0" OC	(1) 16d AT 6" OC	AT 1'-4" OC
W2	15/32" CD-EXT ON (1) SIDE	10d AT 4" OC	10d AT 1'-0" OC	3x	3x	5/8"Ø x 8" EMBED AT 3'-0" OC	(1) 16d AT 4" OC	AT 1'-4" OC
W3	15/32" CD-EXT ON (1) SIDE	10d AT 3" OC	10d AT 1'-0" OC	3x	3x	3/4"Ø x 8" EMBED AT 2'-4" OC	(2) 16d AT 6" OC	AT 8" OC
W4	15/32" CD-EXT ON (1) SIDE	10d AT 2" OC	10d AT 1'-0" OC	3x	3x	3/4"Ø x 8" EMBED AT 2'-0" OC	(2) 16d AT 4" OC	AT 1'-0" OC, EACH SIDE
W5	15/32" CD-EXT ON (2) SIDES	10d AT 4" OC	10d AT 1'-0" OC	3x	3x	3/4"Ø x 8" EMBED AT 1'-4" OC	(2) 16d AT 3" OC	AT 8" OC, EACH SIDE
W6	15/32" CD-EXT ON (2) SIDES	10d AT 3" OC	10d AT 1'-0" OC	3x	3x	3/4"Ø x 8" EMBED AT 1'-4" OC	(2) 16d AT 2 1/2" OC	AT 6" OC, EACH SIDE



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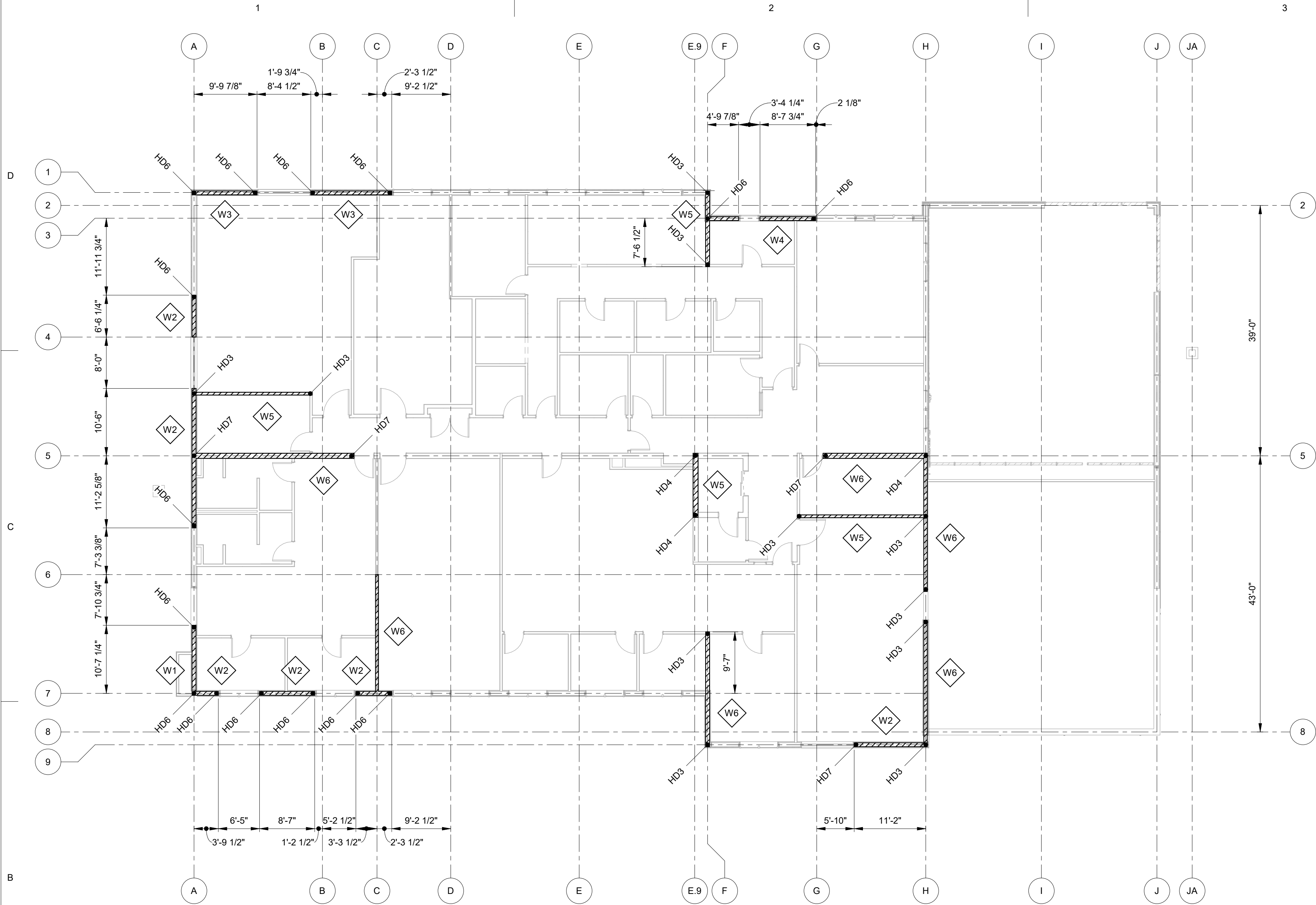
YAKUTAT TLINGIT TRIBE  
YCHC ALTERNATE DESIGN  
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Revisions		
No.	Description	Date
1	REVISIONS	06/18/19

Drawn by DJM	Date 04/29/2019
Checked AKM	Job No. 17341JN

Sheet Contents  
FIRST FLOOR SHEAR WALL  
PLAN

Sheet No.  
**S2.8**



**1 SECOND FLOOR SHEAR WALL PLAN**  
S2.9 SCALE: 3/32" = 1'-0"

HOLDOWN SCHEDULE					
MARK	SIMPSON HOLDOWN (OR APPROVED EQUAL)	MINIMUM CHORD	FASTENERS AT CHORD	ANCHOR BOLT	NOTES
HD1	HD12	6x6	(4) 1"Ø BOLTS	1 1/8"Ø	-
HD2	HD19	6x6	(5) 1"Ø BOLTS	1 1/4"Ø	-
HD3	HDU11-SDS2.5	8x8	(30) 1/4"Ø x 2 1/2" SDS	1"Ø	-
HD4	HSS	6x6	-	(4) 5/8"Ø	-
HD5	(2) HD12	6x6	(4) 1"Ø BOLTS AT EACH HD12	(2) 1 1/8"Ø	SEE DETAIL 8/S3.1
HD6	MSTC40	(2) 2x	(28) 0.148"Ø x 3 1/4" SD	-	STRAP
HD7	MSTC66	(2) 2x	(64) 0.148"Ø x 3 1/4" SD	-	STRAP

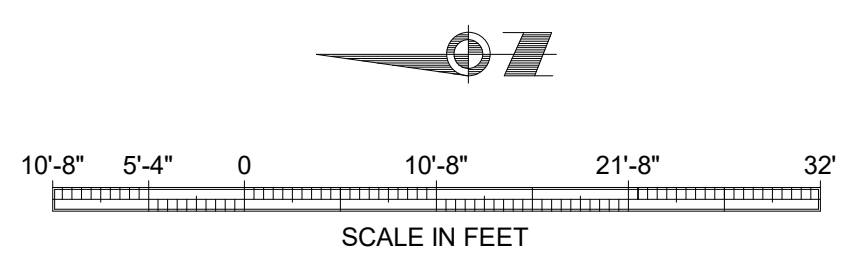
**HOLDOWN SCHEDULE NOTES**

1. PROVIDE PANEL EDGE NAILING PER SHEAR WALL SCHEDULE AT HOLDDOWN STUDS AND AT PANEL EDGES.
2. HOLDOWNS LOCATED PER PLAN.
3. HOLDOWNS SHALL BE SIMPSON STRONG TIE OR APPROVED EQUAL.
4. SEE DETAILS FOR HOLDOWN CONFIGURATION.

**SHEAR WALL SCHEDULE NOTES**

1. INSTALL PANELS EITHER HORIZONTALLY OR VERTICALLY.
2. WHERE SHEATHING IS APPLIED ON BOTH SIDES OF WALL, PANEL EDGE JOINTS ON 2x FRAMING SHALL BE STAGGERED SO THAT JOINTS ON THE OPPOSITE SIDES ARE NOT LOCATED ON THE SAME STUD.
3. BLOCKING IS REQUIRED AT ALL PANEL EDGES.
4. PROVIDE SHEAR WALL SHEATHING AND NAILING FOR THE ENTIRE LENGTH OF THE WALLS INDICATED ON THE PLANS. SEE PLANS FOR HOLDOWN REQUIREMENTS.
5. SHEATHING EDGE NAILING IS REQUIRED AT ALL HOLDOWN POSTS.
6. INTERMEDIATE FRAMING MEMBERS SHALL BE 2x.
7. FRAMING CLIPS: SIMPSON STRONG TIE A35 OR LTP5 EQUIV.
8. PROVIDE DOUBLE JOIST, RIM OR EQUAL WHERE PLATE ATTACHMENT SPECIFIES (2) ROWS OF NAILS.
9. ANCHOR BOLTS SHALL BE PROVIDED WITH STEEL PLATE WASHERS 1/4"X3"X3".
10. PROVIDE HOT-DIPPED GALVANIZED NAILS AND CONNECTOR PLATES FOR ALL CONNECTORS IN CONTACT WITH PRESSURE TREATED FRAMING MEMBERS.
11. OSB SHEATHING CAN BE USED IF APPROVED BY ENGINEER OF RECORD.
12. (2) 2X STUDS NAILED TOGETHER MAY BE USED IN PLACE OF A SINGLE 3X STUD. (2) 2X STUDS SHALL BE JOINED TOGETHER BY USING PLATE NAILING REQUIREMENTS.

SHEAR WALL SCHEDULE								
WALL TYPE	WALL APA-RATED SHEATHING [1] [2] [4] [12]	NAIL SIZE AND SPACING AT EDGES [4] [5]	NAIL SIZE AND SPACING AT INTERMEDIATE FRAMING MEMBERS [6]	STUD AND BLOCKING SIZE AD ADJOINING EDGES [3] [4]	SILL PLATE SIZE [10]	ANCHOR BOLT TO CONCRETE BELOW [10]	SILL PLATE NAILING	RIM JOIST OR BLOCKING CONNECTION TO TOP PLATE [7] SPACING
W1	15/32" CD-EXT ON (1) SIDE	8d AT 6" OC	8d AT 1'-0" OC	2x	2x	5/8"Ø x 8" EMBED AT 4'-0" OC	(1) 16d AT 6" OC	AT 1'-4" OC
W2	15/32" CD-EXT ON (1) SIDE	10d AT 4" OC	10d AT 1'-0" OC	3x	3x	5/8"Ø x 8" EMBED AT 3'-0" OC	(1) 16d AT 4" OC	AT 1'-4" OC
W3	15/32" CD-EXT ON (1) SIDE	10d AT 3" OC	10d AT 1'-0" OC	3x	3x	3/4"Ø x 8" EMBED AT 2'-4" OC	(2) 16d AT 6" OC	AT 8" OC
W4	15/32" CD-EXT ON (1) SIDE	10d AT 2" OC	10d AT 1'-0" OC	3x	3x	3/4"Ø x 8" EMBED AT 2'-0" OC	(2) 16d AT 4" OC	AT 1'-0" OC, EACH SIDE
W5	15/32" CD-EXT ON (2) SIDES	10d AT 4" OC	10d AT 1'-0" OC	3x	3x	3/4"Ø x 8" EMBED AT 1'-4" OC	(2) 16d AT 3" OC	AT 8" OC, EACH SIDE
W6	15/32" CD-EXT ON (2) SIDES	10d AT 3" OC	10d AT 1'-0" OC	3x	3x	3/4"Ø x 8" EMBED AT 1'-4" OC	(2) 16d AT 2 1/2" OC	AT 6" OC, EACH SIDE



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FOUNDATION PACKAGE  
YAKUTAT, ALASKA

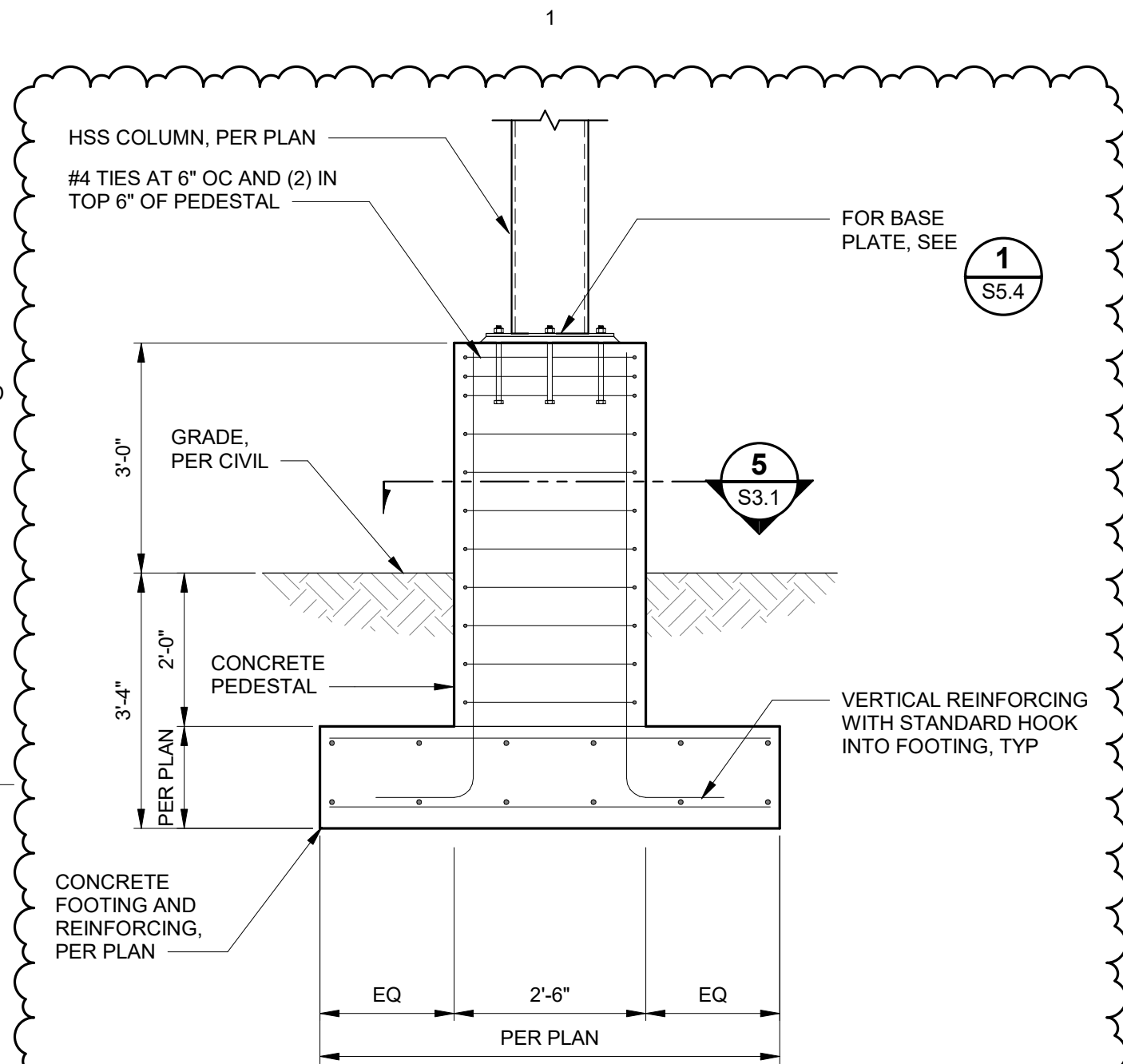
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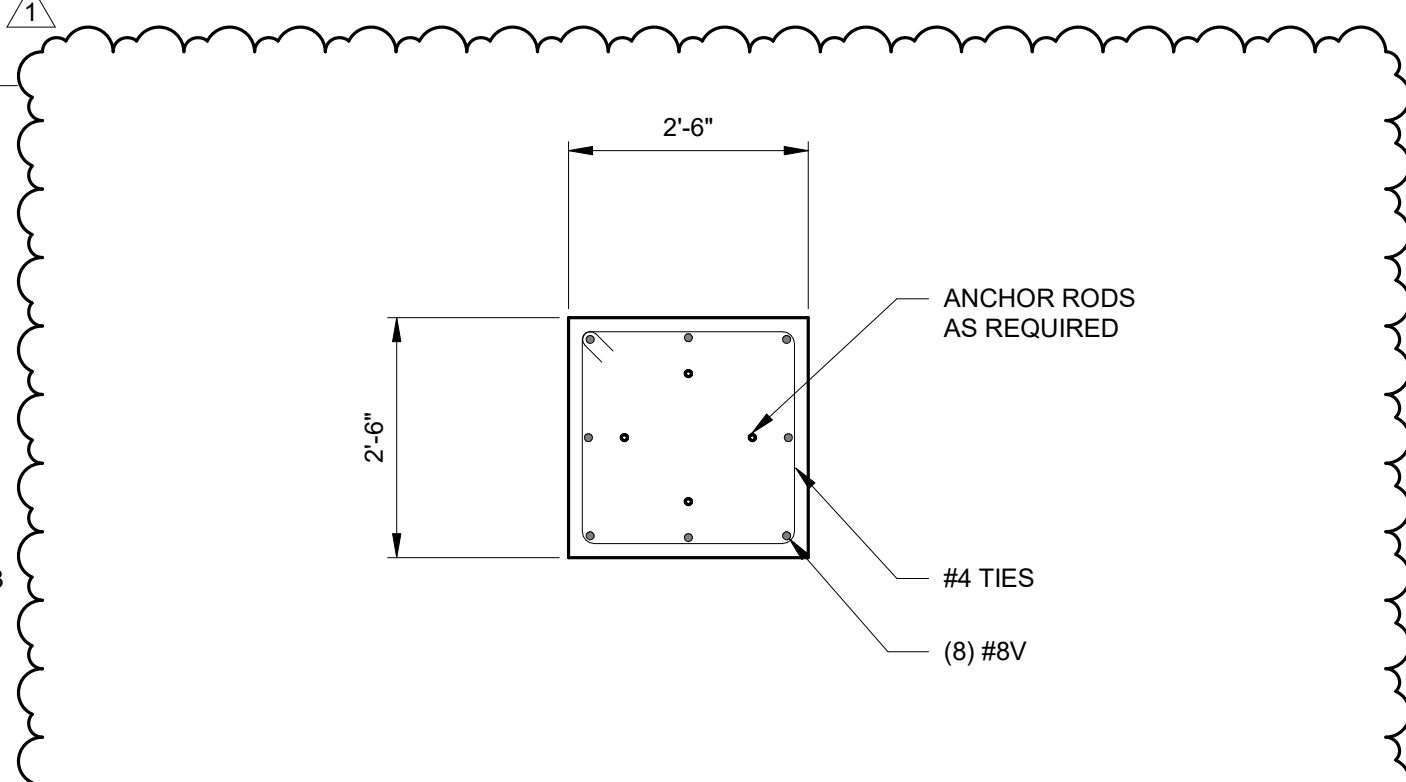
Sheet Contents  
SECOND FLOOR SHEAR WALL  
PLAN

Sheet No.  
**S2.9**

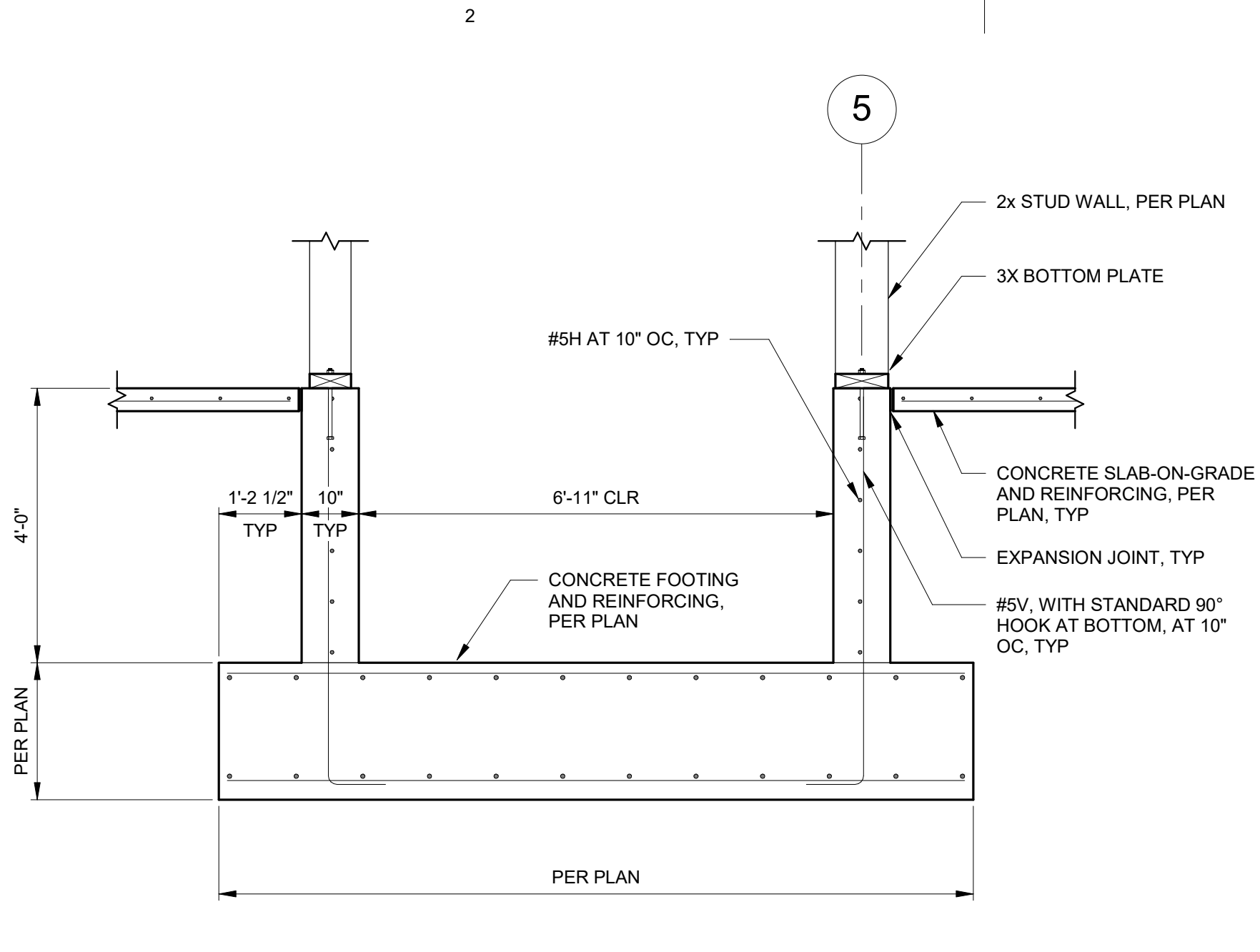
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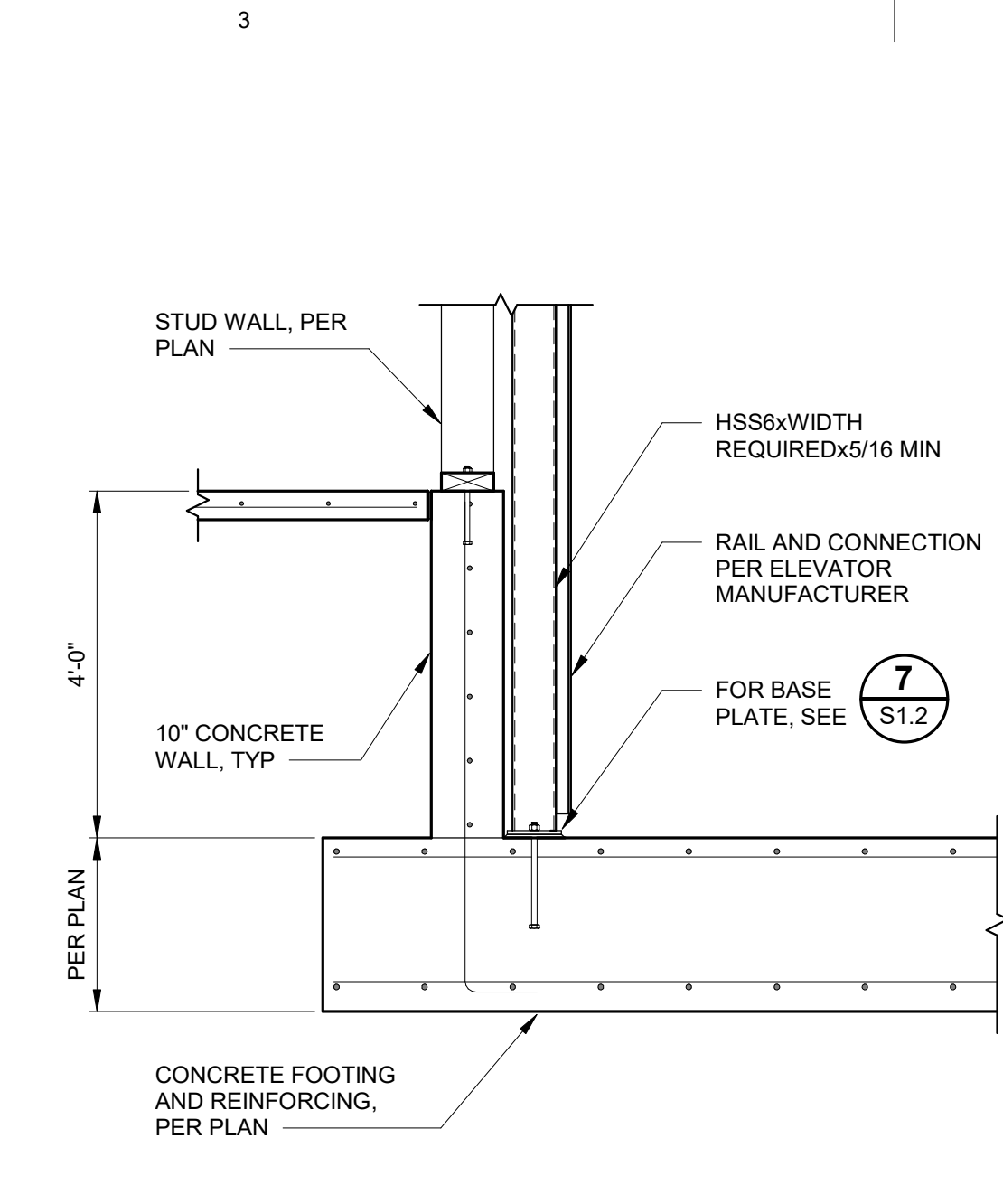
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S3.1 SCALE: 1/2" = 1'-0"



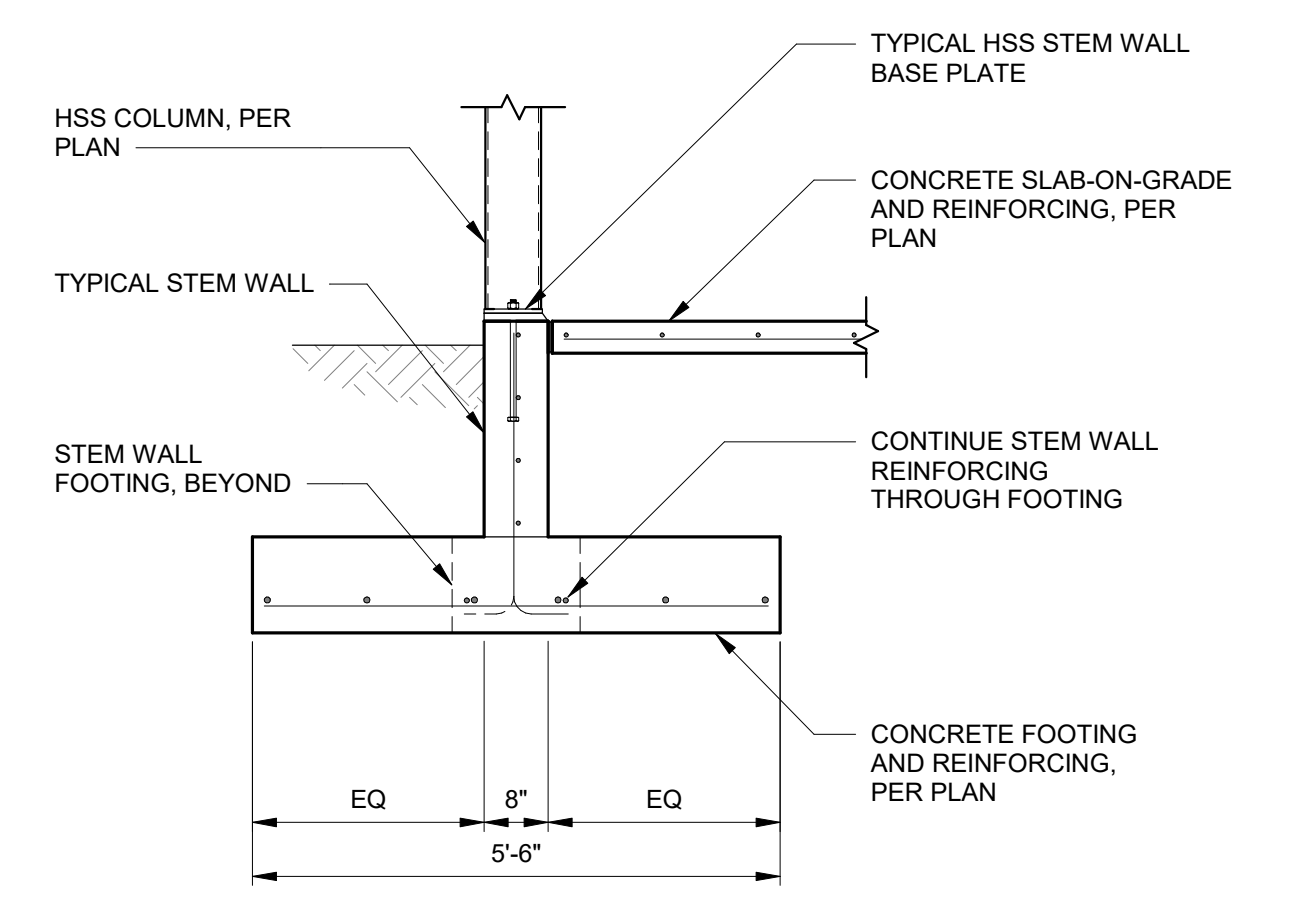
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S3.1 SCALE: 1/2" = 1'-0"



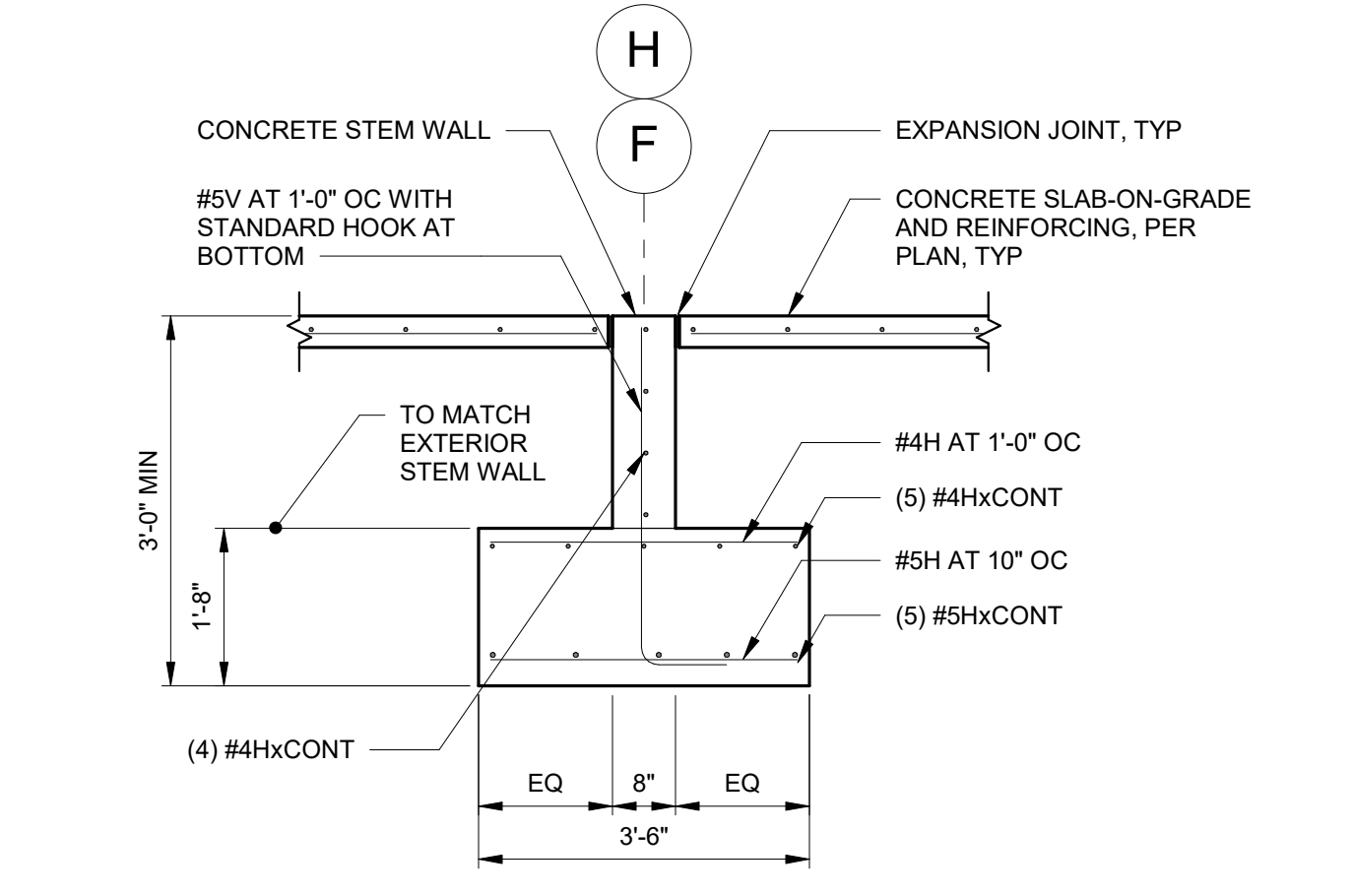
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S3.1 SCALE: 1/2" = 1'-0"



**3 FOUNDATION SECTION AT ELEVATOR PIT**  
S3.1 SCALE: 1/2" = 1'-0"

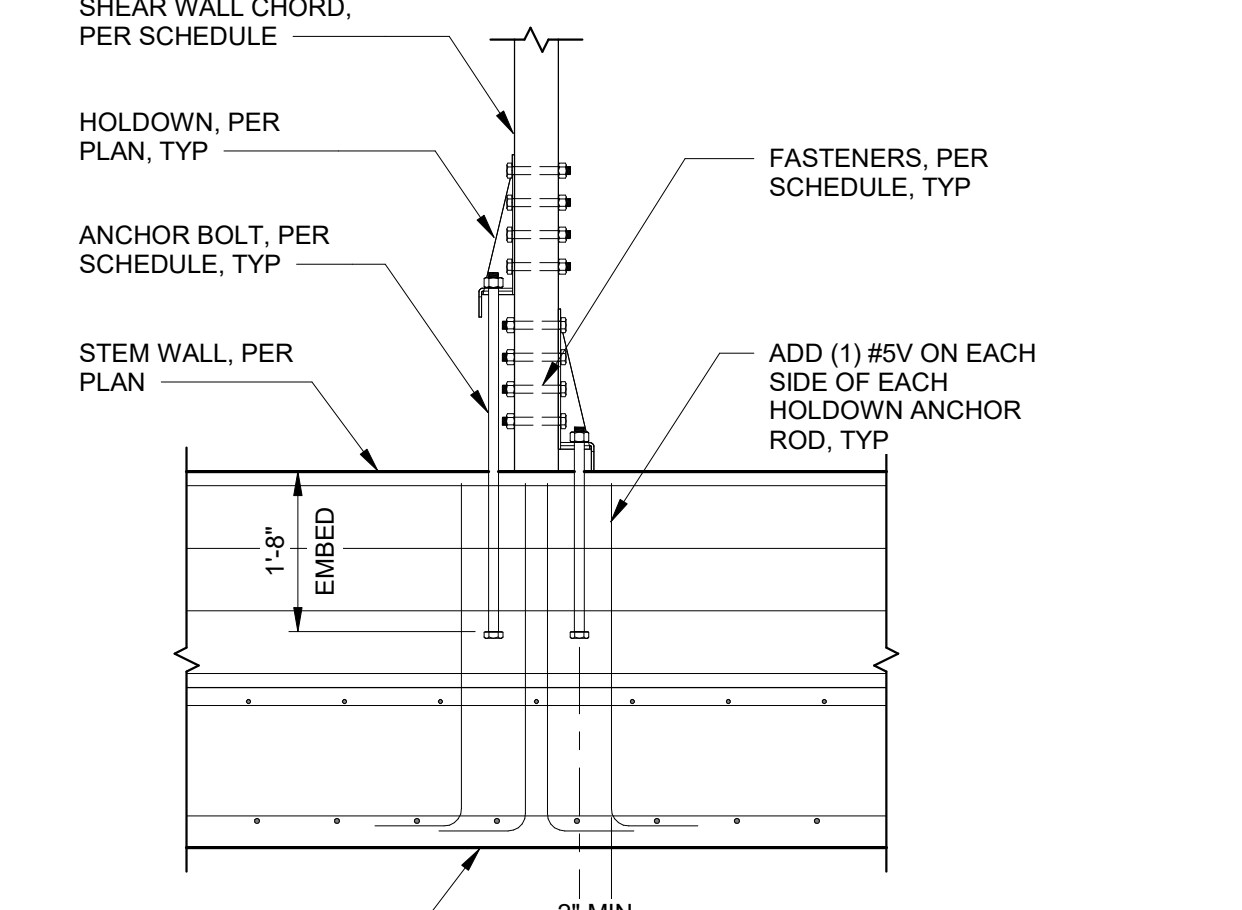


**6 EXTERIOR SPREAD FOOTING SECTION**  
S3.1 SCALE: 1/2" = 1'-0"



**7 INTERIOR STEM WALL SECTION**  
S3.1 SCALE: 1/2" = 1'-0"

NOTES:  
1. WHERE EXPOSED CONCRETE SLAB IS LOCATED AT INTERIOR STEM WALL, NOTCH STEM WALL TO KEEP SLAB CONTINUOUS PER 9/S1.2. LOCATE CONTROL JOINT ALONG ONE SIDE OF STEM WALL.



**8 HOLDOWN DETAIL AT HD5**  
S3.1 SCALE: 1/2" = 1'-0"

NOTES:  
1. TYPICAL VERTICAL STEM WALL REINFORCING NOT SHOWN FOR CLARITY.

Plot Date Time: 6/18/2019 5:00:38 PM

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SE 13980  
4/29/19

YAKUTAT TLINGIT TRIBE  
YCHC ALTERNATE DESIGN  
FOUNDATION PACKAGE  
YAKUTAT, ALASKA

No.	Description	Date
1	REVISIONS	06/18/19

Drawn by DJM	Date 04/29/2019
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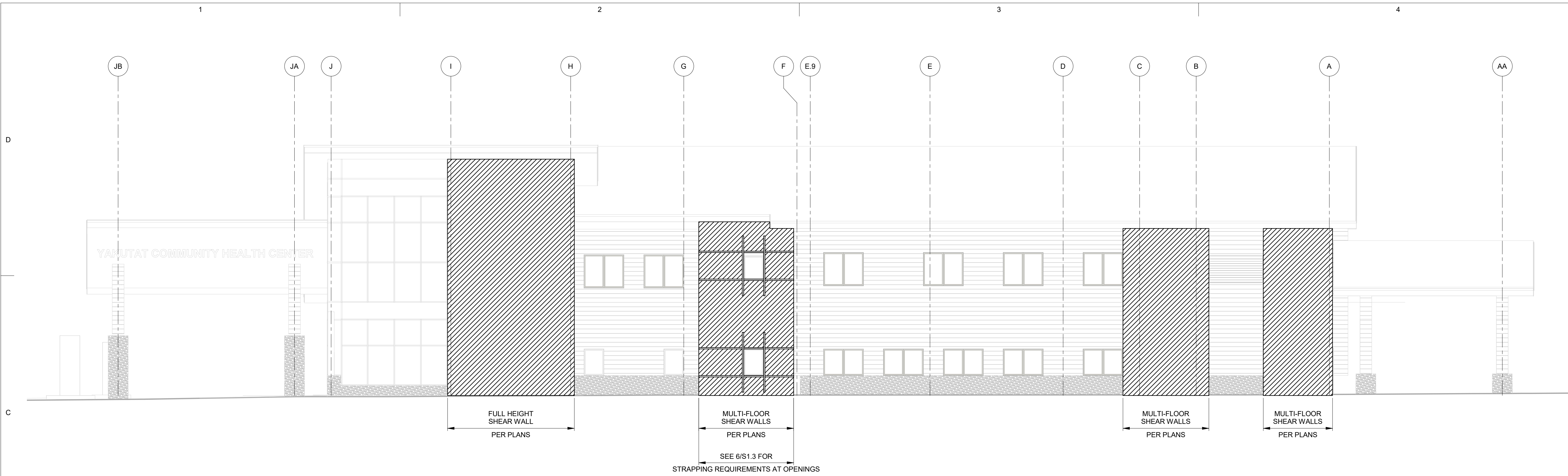
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FOUNDATION SECTIONS AND  
DETAILS

Sheet No.  
**S3.1**

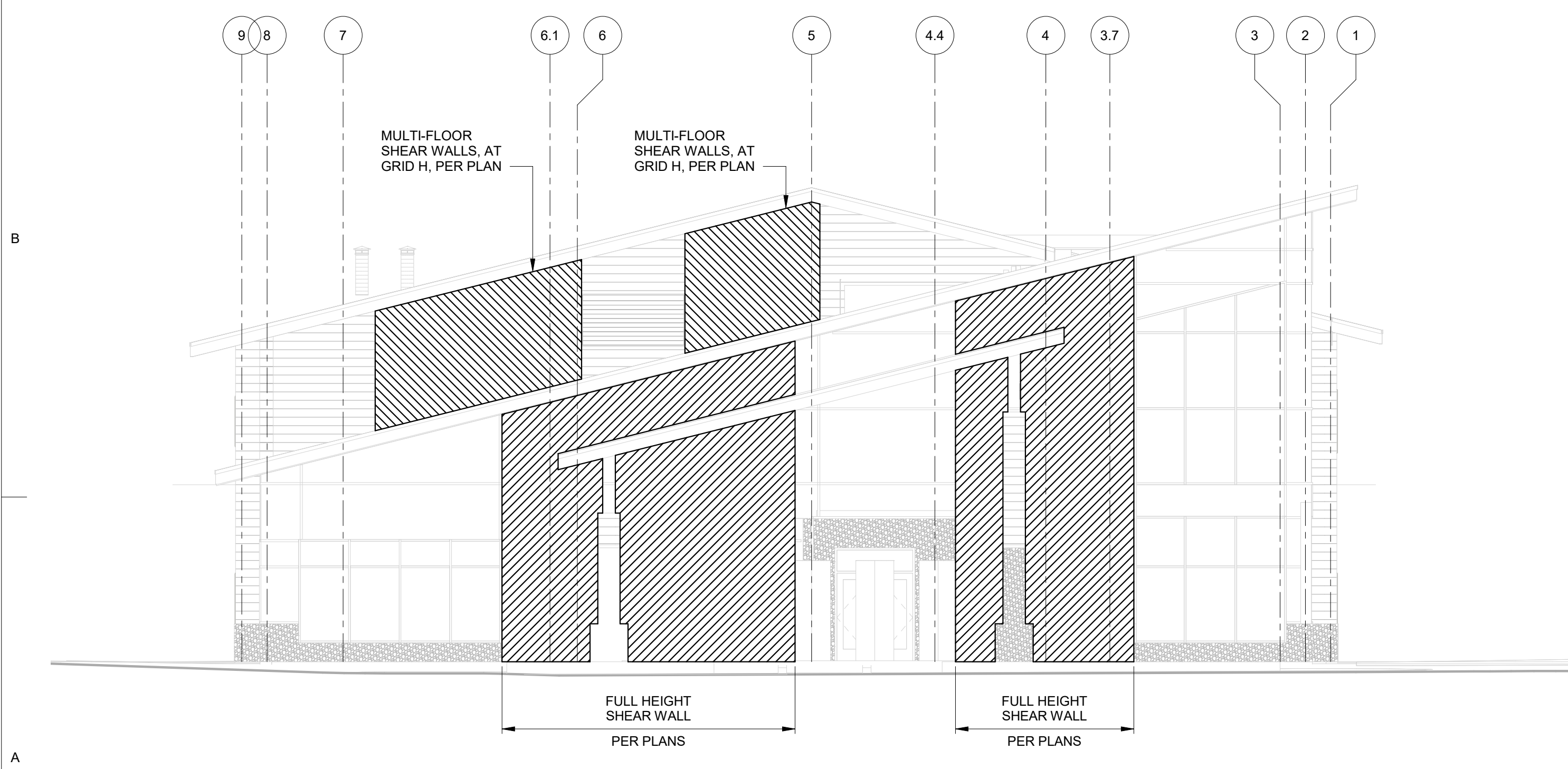


BIM 360://17046.01 Yakutat Community Health Center/PDC-STR-17341.JN.rvt

Plot Date Time: 6/18/2019 5:00:41 PM



**1** SHEAR WALL ELEVATION - EAST  
S4.1 SCALE: 1/8" = 1'-0"



**2** SHEAR WALL ELEVATION - SOUTH  
S4.1 SCALE: 1/8" = 1'-0"

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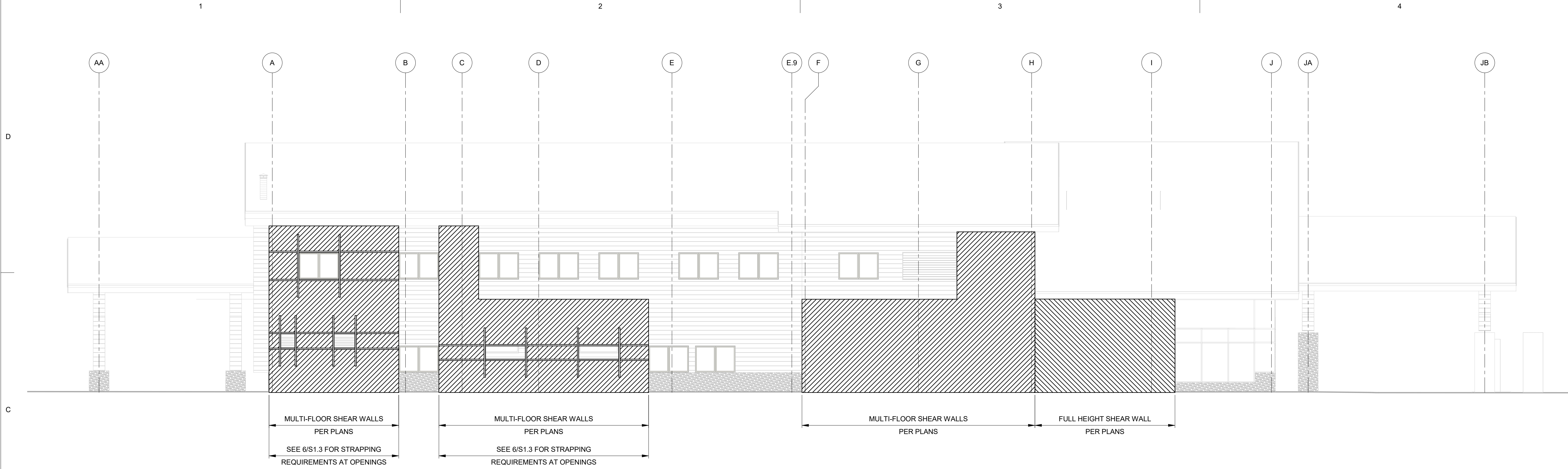
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Sheet Contents  
SHEAR WALL ELEVATIONS (1 OF 2)

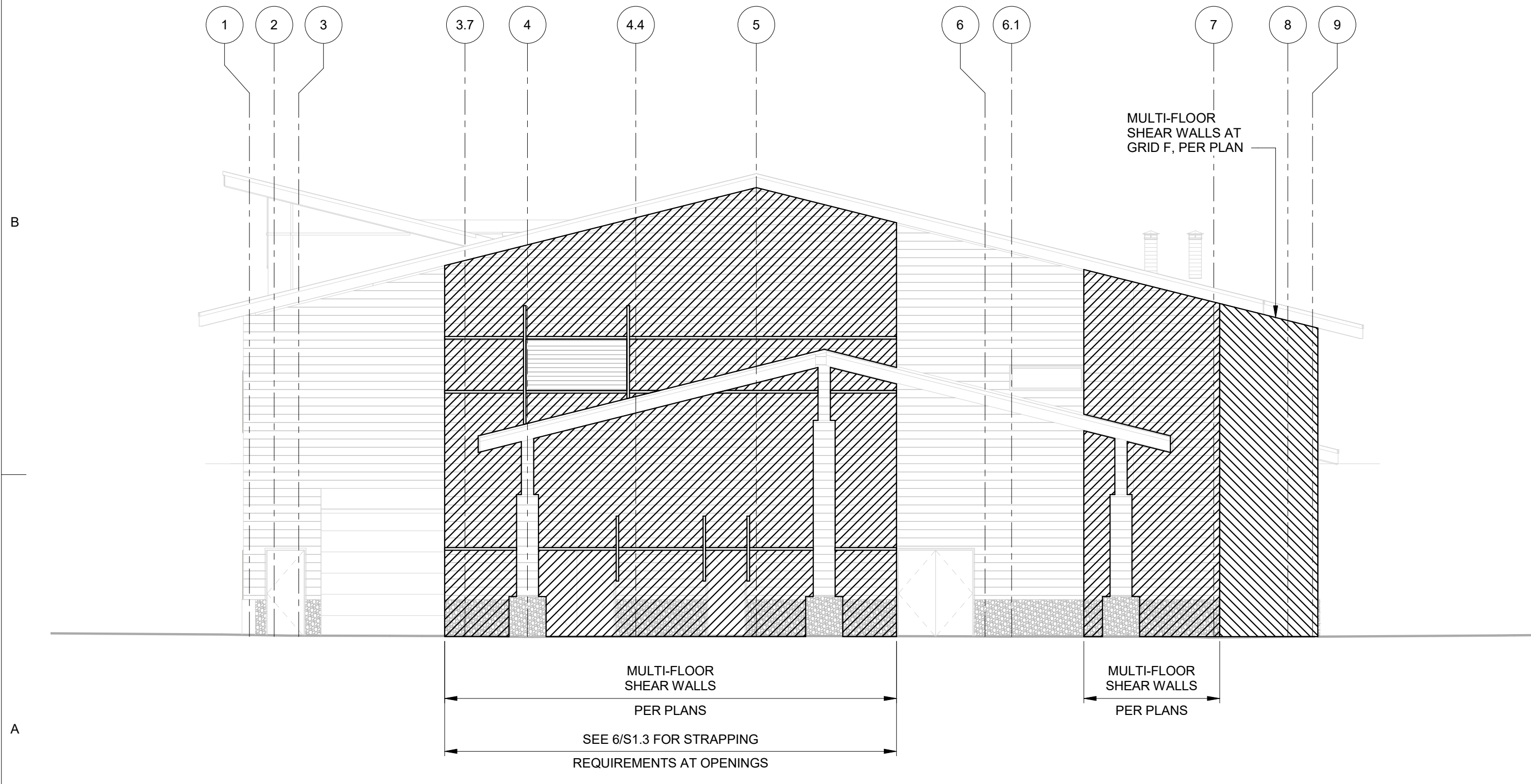
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**1** SHEAR WALL ELEVATION - WEST  
S4.2 SCALE: 1/8" = 1'-0"



**2** SHEAR WALL ELEVATION - NORTH  
S4.2 SCALE: 1/8" = 1'-0"

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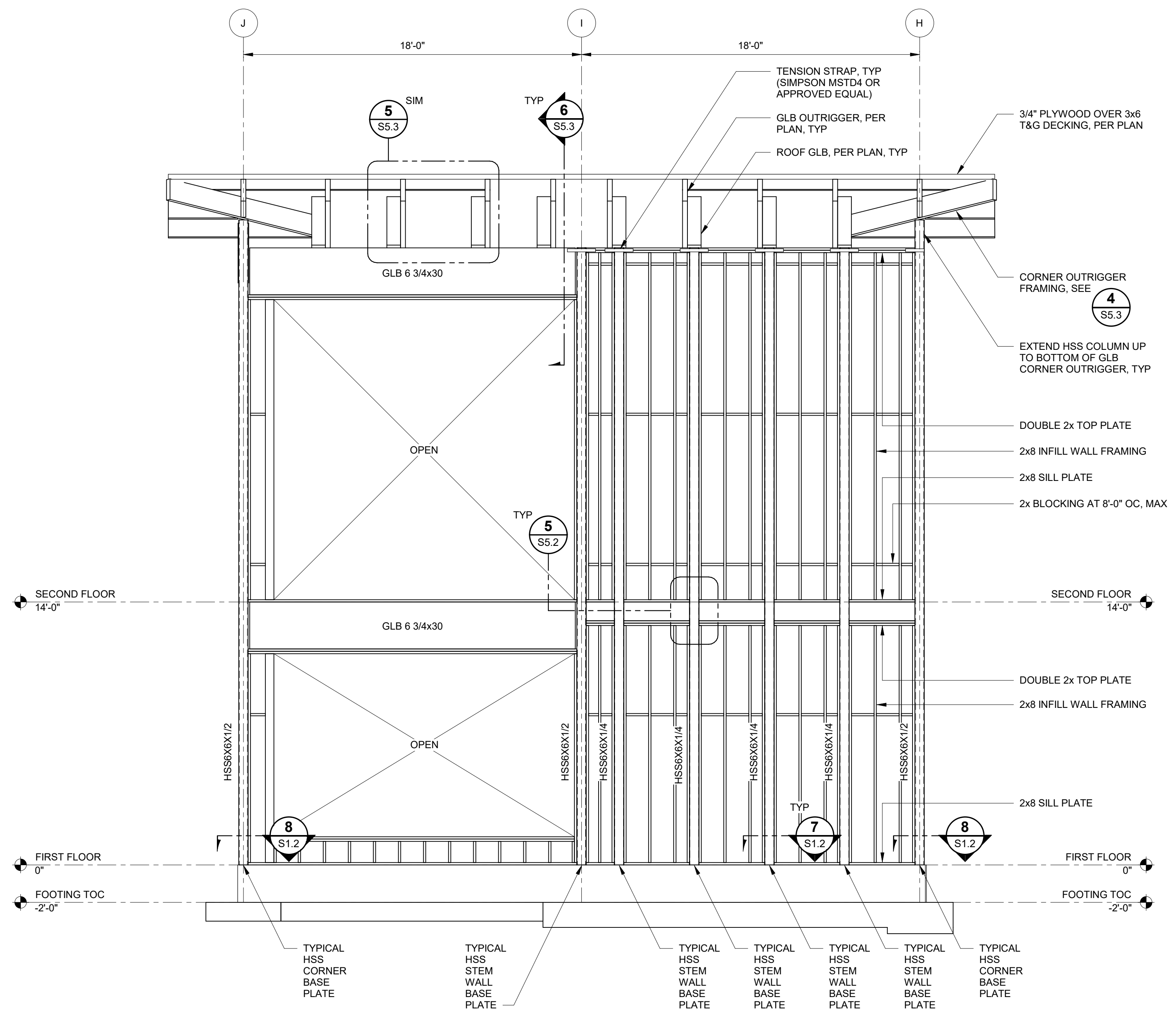
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Revisions		
No.	Description	Date

Drawn by DJM	Date 06/18/2019
Checked AKM	Job No. 17341JN

Sheet Contents  
SHEAR WALL ELEVATIONS (2 OF 2)

Sheet No.  
**S4.2**



**1 PARTIAL FRAMING ELEVATION AT GRID LINE 2**  
 S4.3 SCALE: 1/4" = 1'-0"

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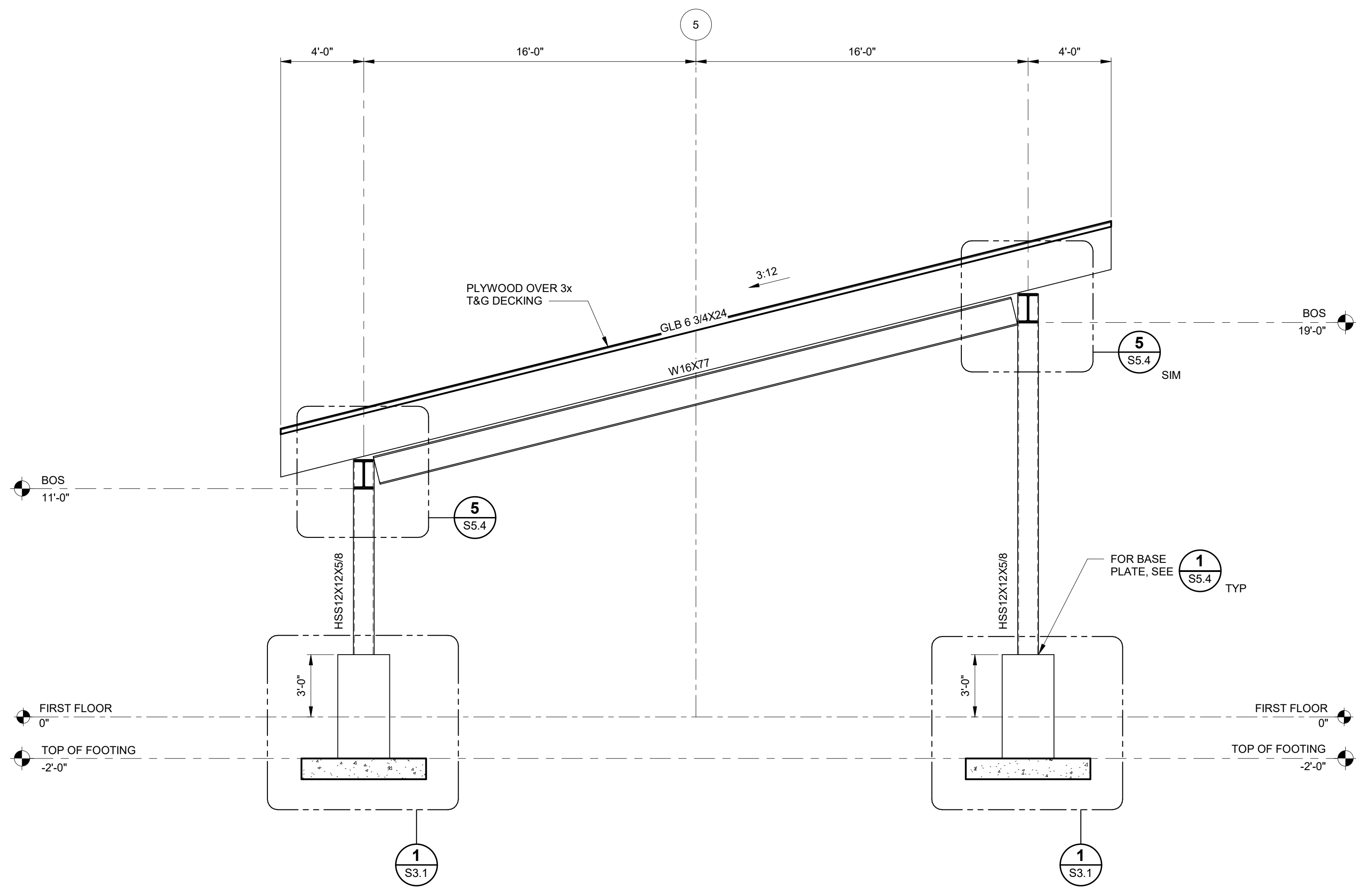
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No.	Description	Date

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Checked AKM	Job No. 17341JN

Sheet Contents FRAMING ELEVATIONS (1 OF 3)
---

Sheet No.  
**S4.3**



**1** FRAMING ELEVATION AT SOUTH CANOPY  
 S4.4 SCALE: 1/4" = 1'-0"

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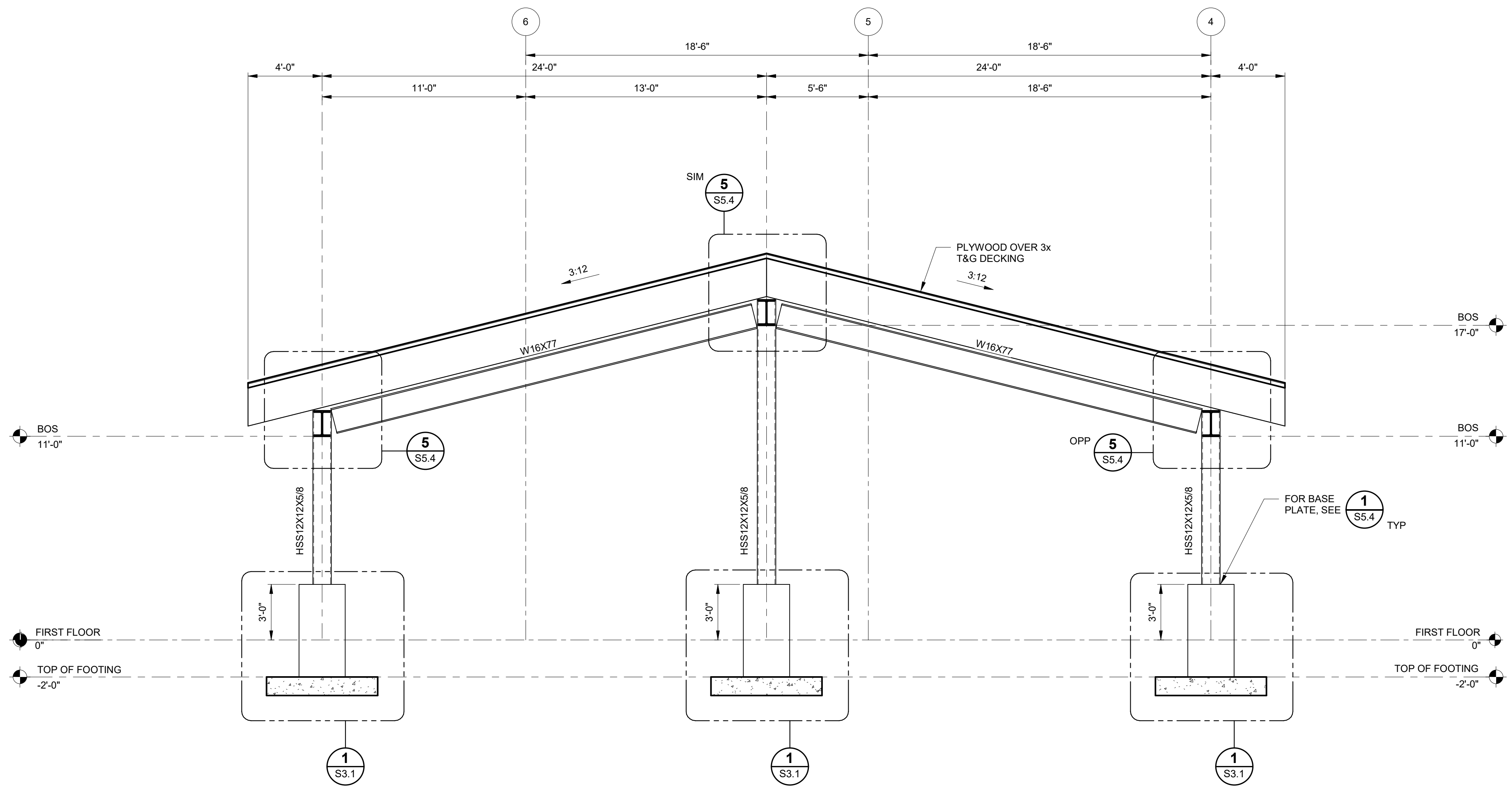
Revisions		
No.	Description	Date

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Checked AKM	Job No. 17341JN

Sheet Contents FRAMING ELEVATIONS (2 OF 3)
---

Sheet No.  
**S4.4**

BIM 360://77046.01 Yakutat Community Health Center/PDC-STR-17341.JN.rvt



**1** FRAMING ELEVATION AT NORTH CANOPY  
 S4.5 SCALE: 1/4" = 1'-0"

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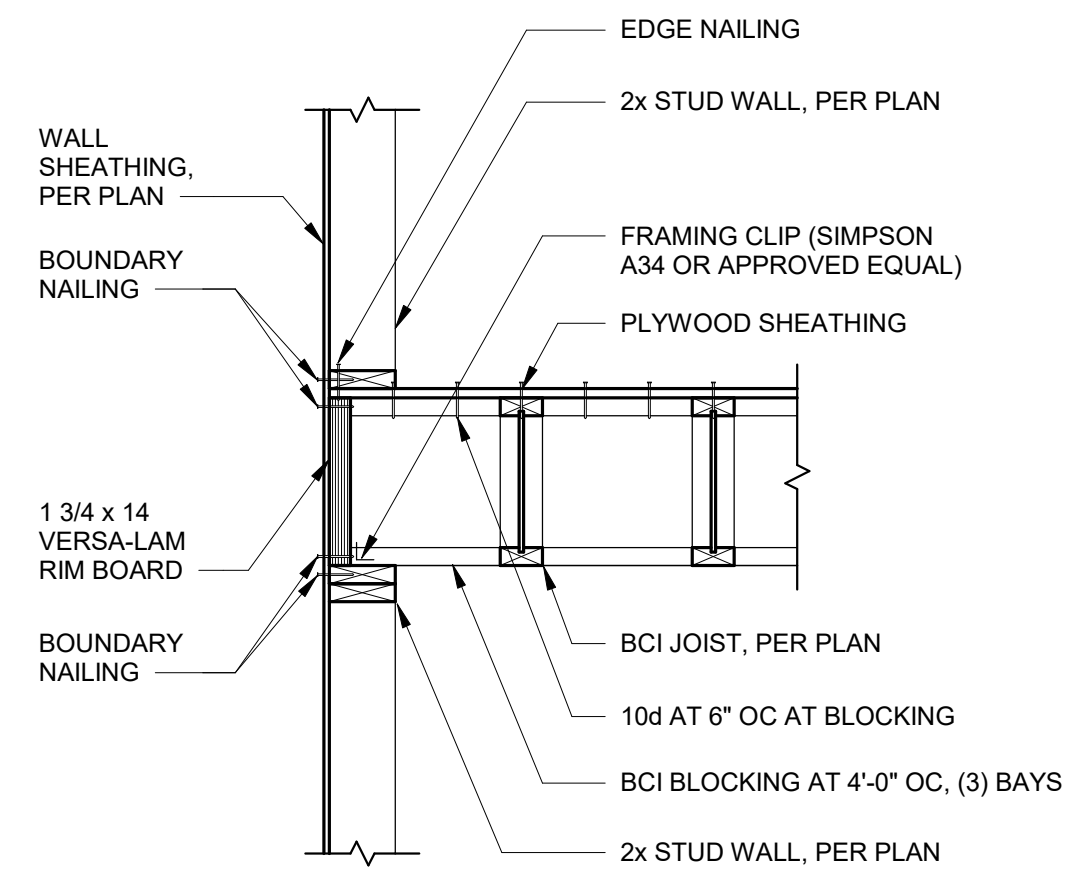
Revisions		
No.	Description	Date

Drawn by DJM	Date 06/18/2019
Checked AKM	Job No. 17341JN

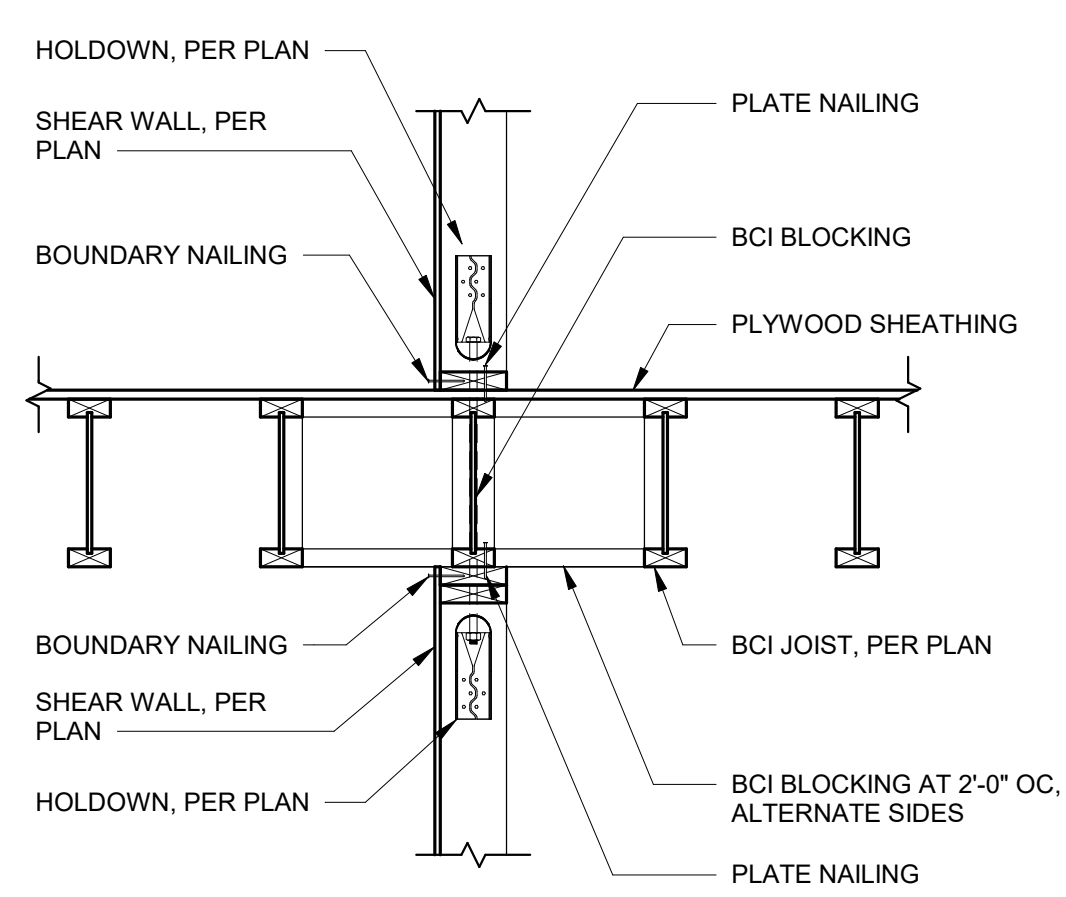
Sheet Contents FRAMING ELEVATIONS (3 OF 3)
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Sheet No.  
**S4.5**

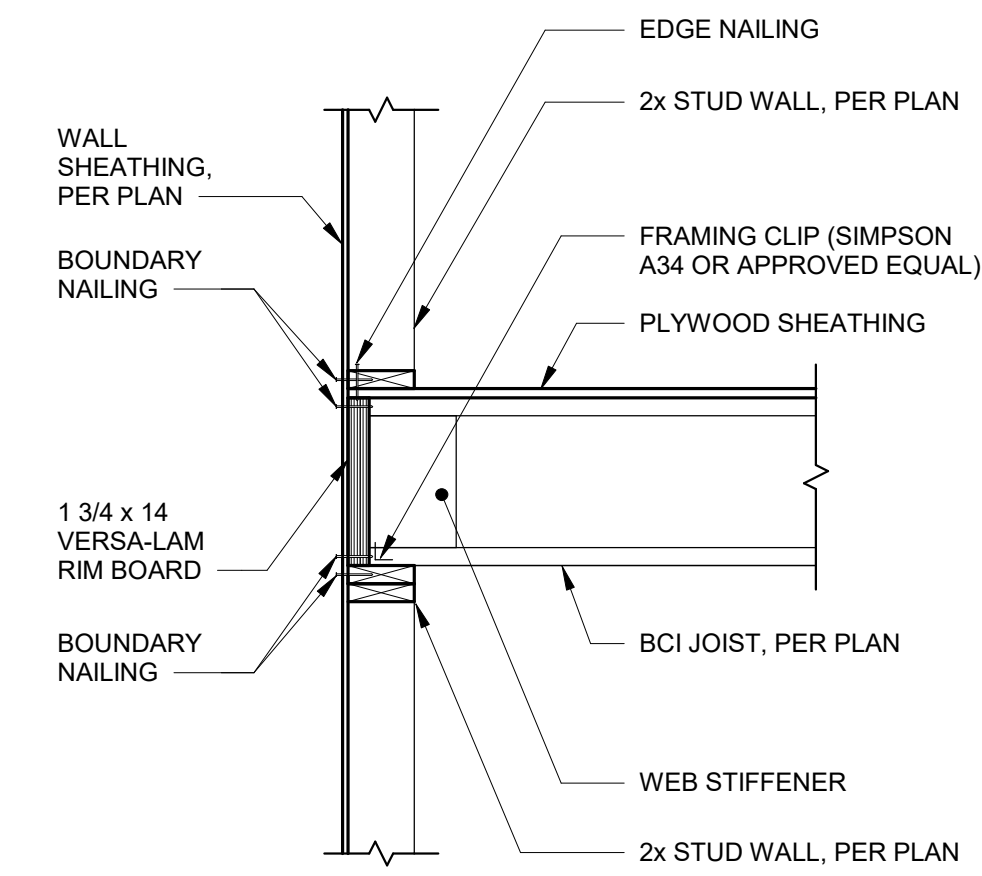
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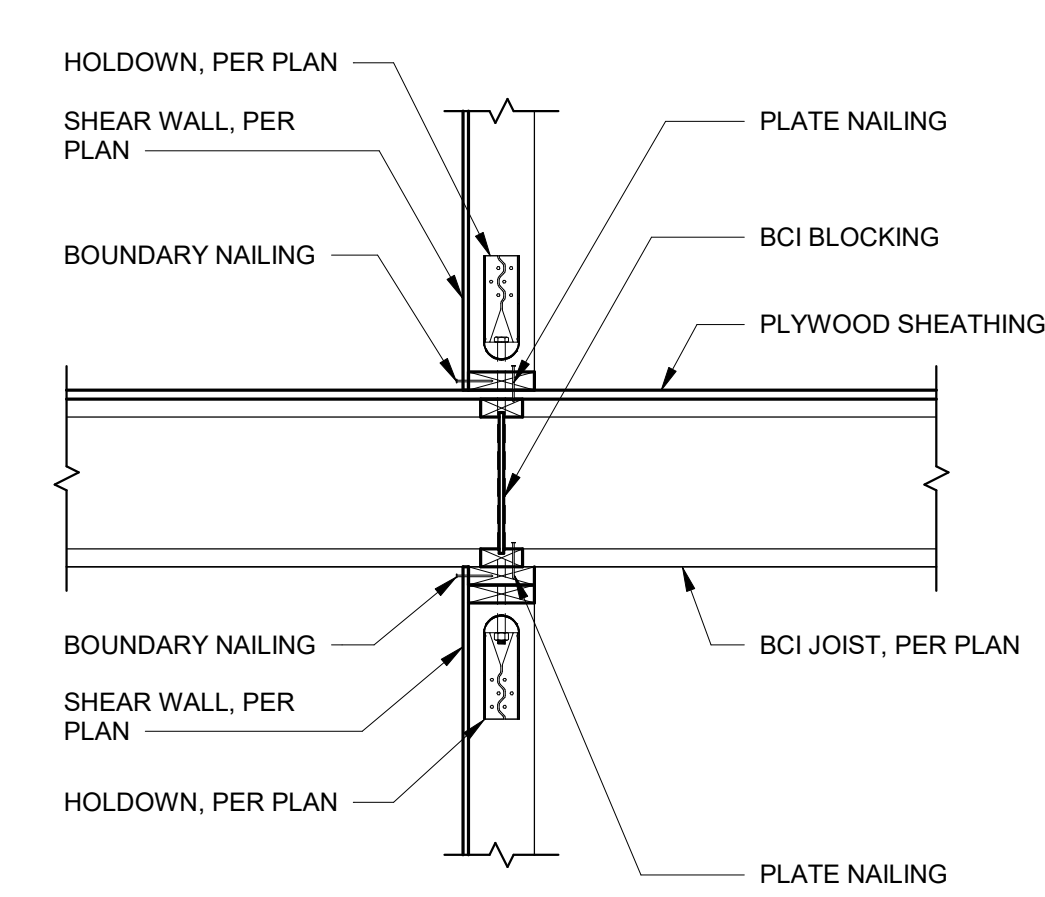
**1** EXTERIOR SHEAR WALL PARALLEL TO JOISTS  
S5.1 SCALE: 3/4" = 1'-0"



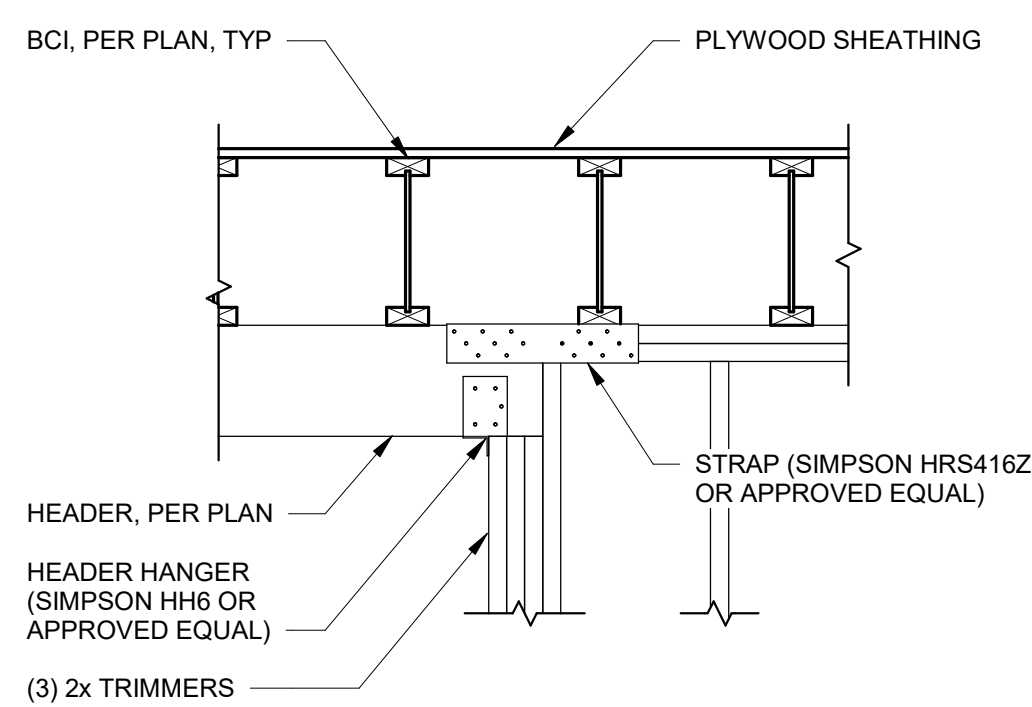
**2** INTERIOR SHEAR WALL PARALLEL TO JOISTS  
S5.1 SCALE: 3/4" = 1'-0"



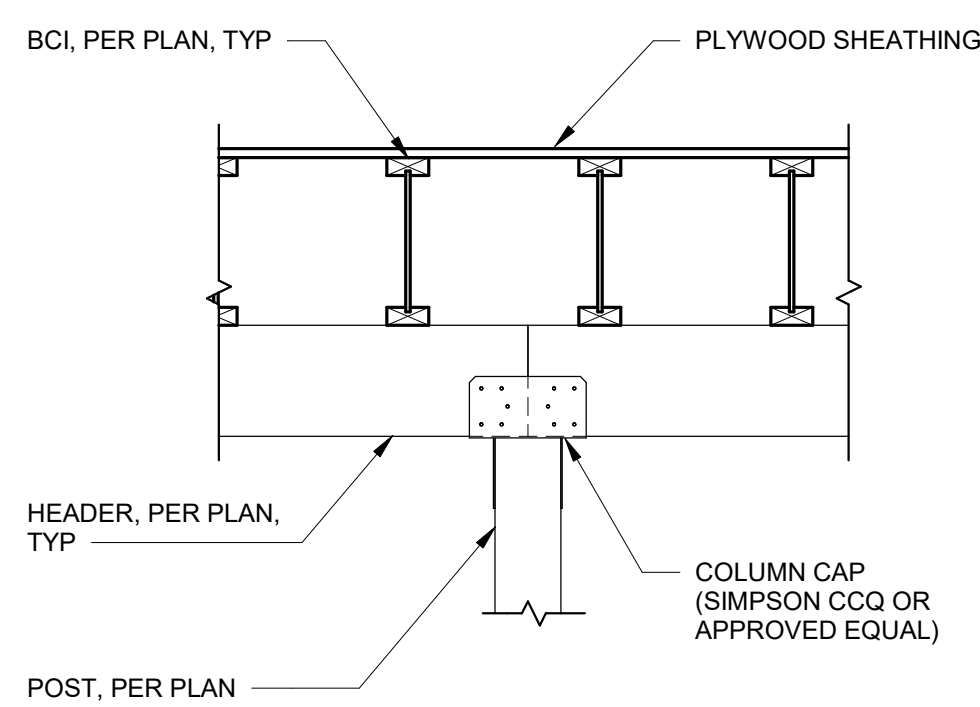
**3** EXTERIOR SHEAR WALL PERPENDICULAR TO JOISTS  
S5.1 SCALE: 3/4" = 1'-0"



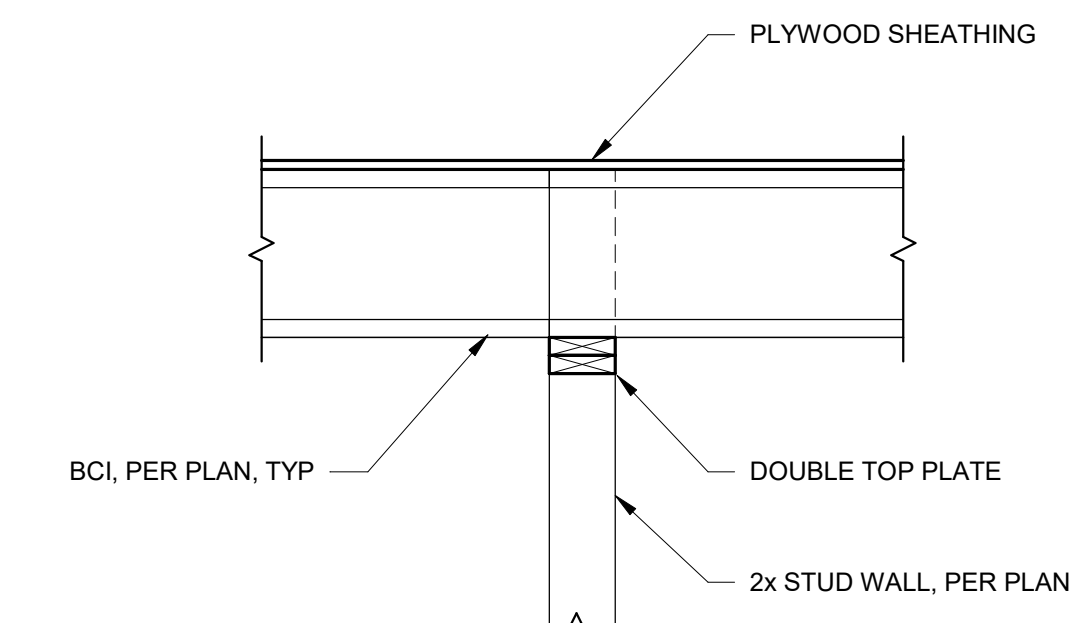
**4** INTERIOR SHEAR WALL PERPENDICULAR TO JOISTS  
S5.1 SCALE: 3/4" = 1'-0"



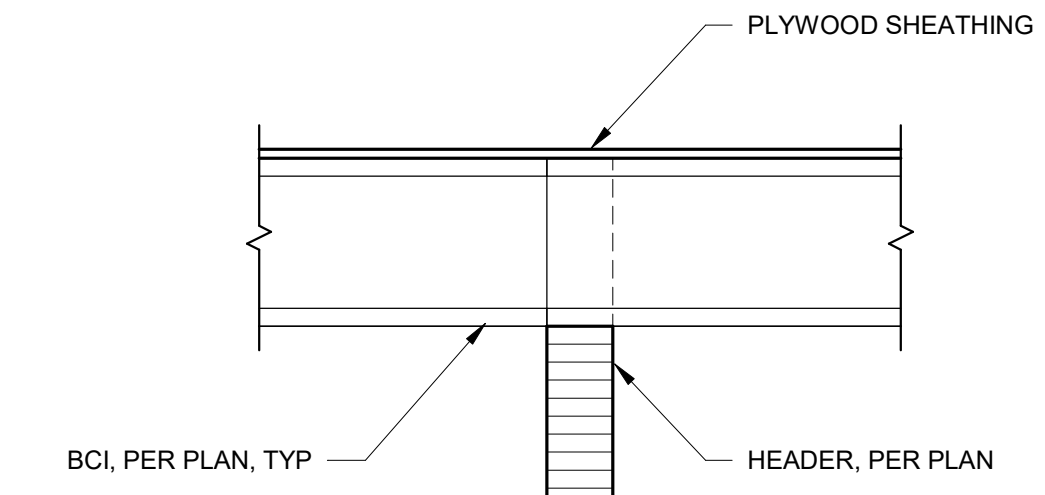
**5** HEADER TO WALL  
S5.1 SCALE: 3/4" = 1'-0"



**6** HEADERS TO POST  
S5.1 SCALE: 3/4" = 1'-0"



**7** INTERIOR WALL PERPENDICULAR TO JOISTS  
S5.1 SCALE: 3/4" = 1'-0"



**8** HEADER PERPENDICULAR TO JOISTS  
S5.1 SCALE: 3/4" = 1'-0"

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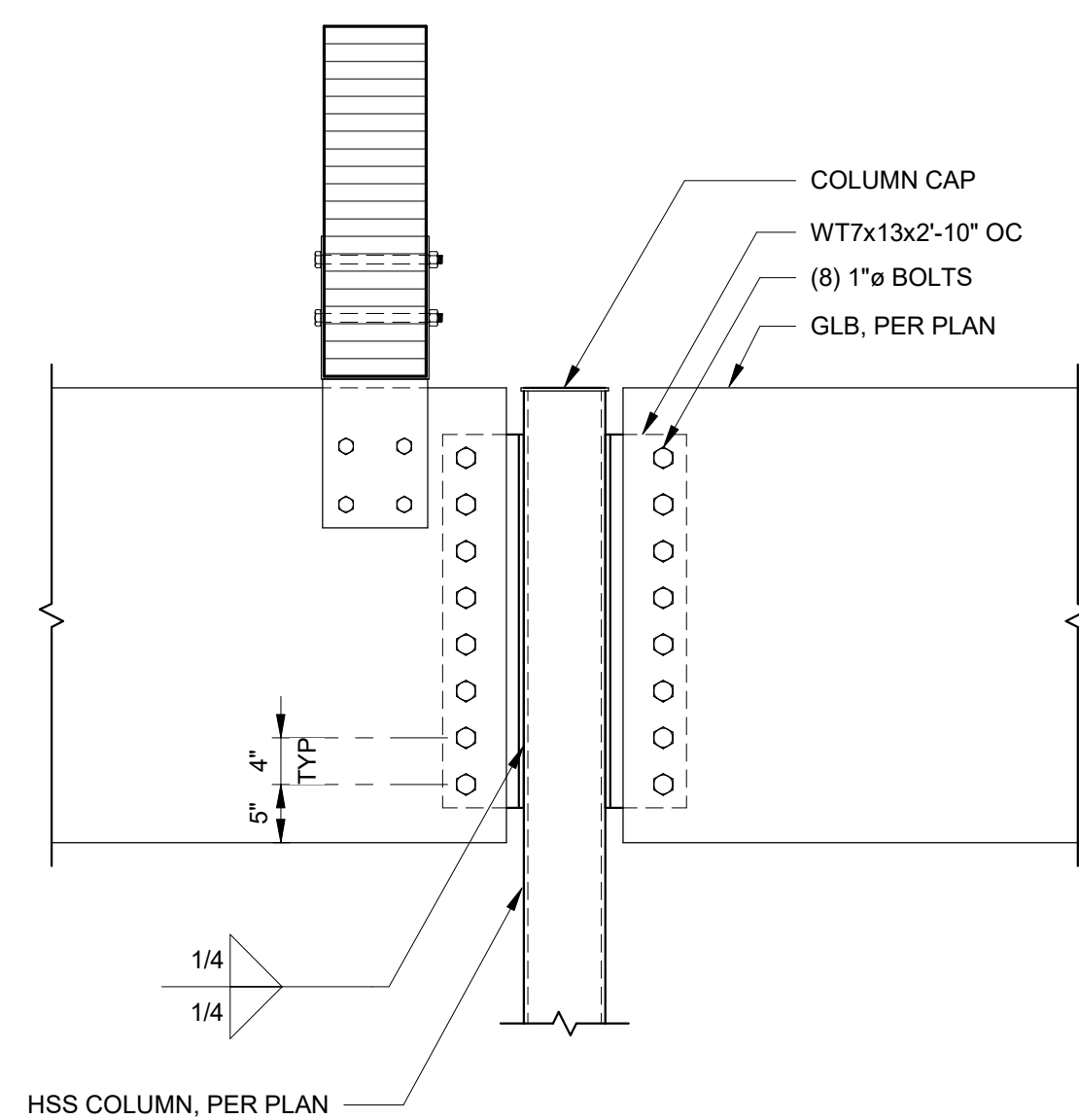
Revisions		
No.	Description	Date

Drawn by DJM	Date 06/18/2019
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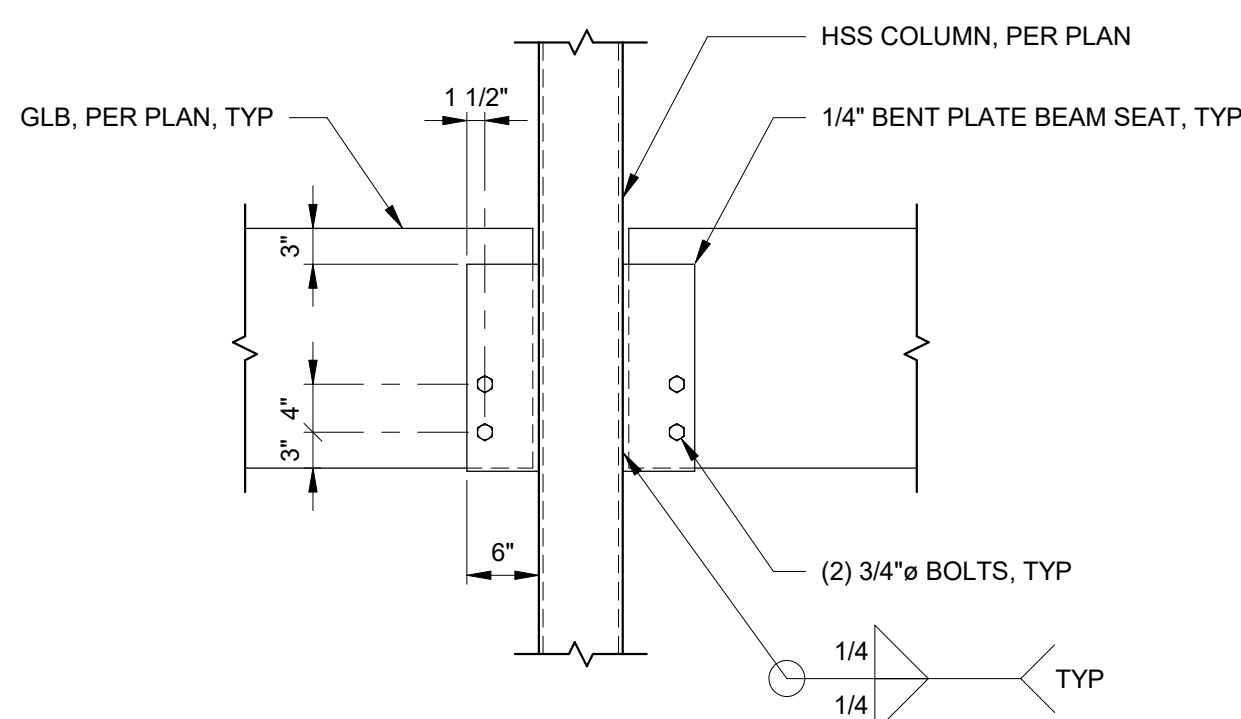
Sheet Contents  
FRAMING SECTIONS AND  
DETAILS (1 OF 4)

Sheet No.  
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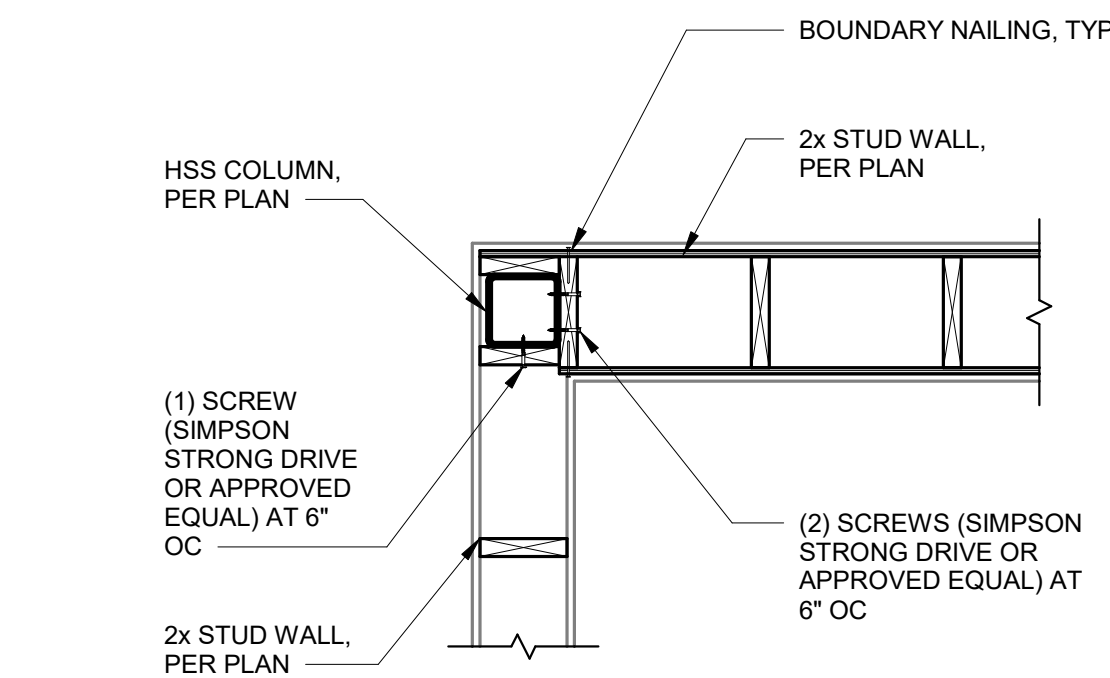
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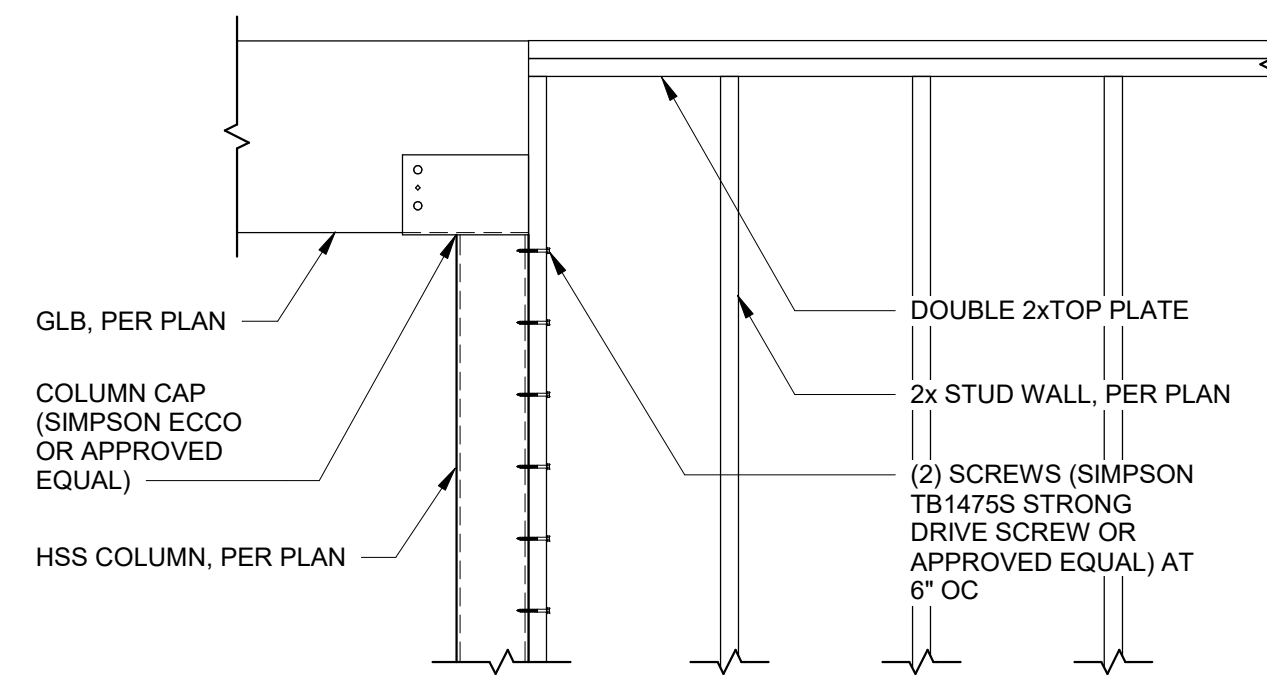
**1** GLB TO COLUMN TEE AT ROOF  
S5.2 SCALE: 3/4" = 1'-0"



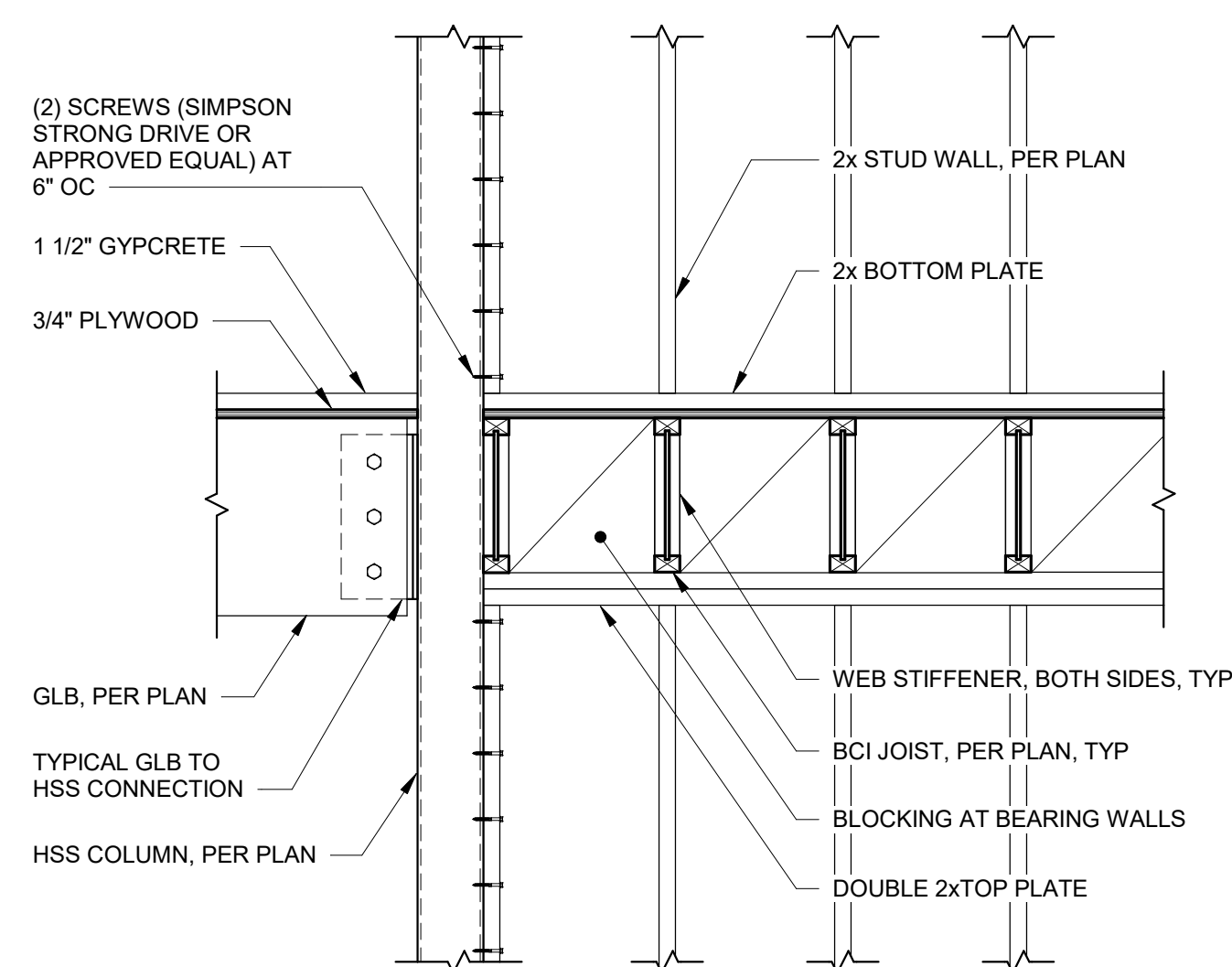
**5** GLB TO COLUMN SADDLE AT MID-HEIGHT  
S5.2 SCALE: 3/4" = 1'-0"



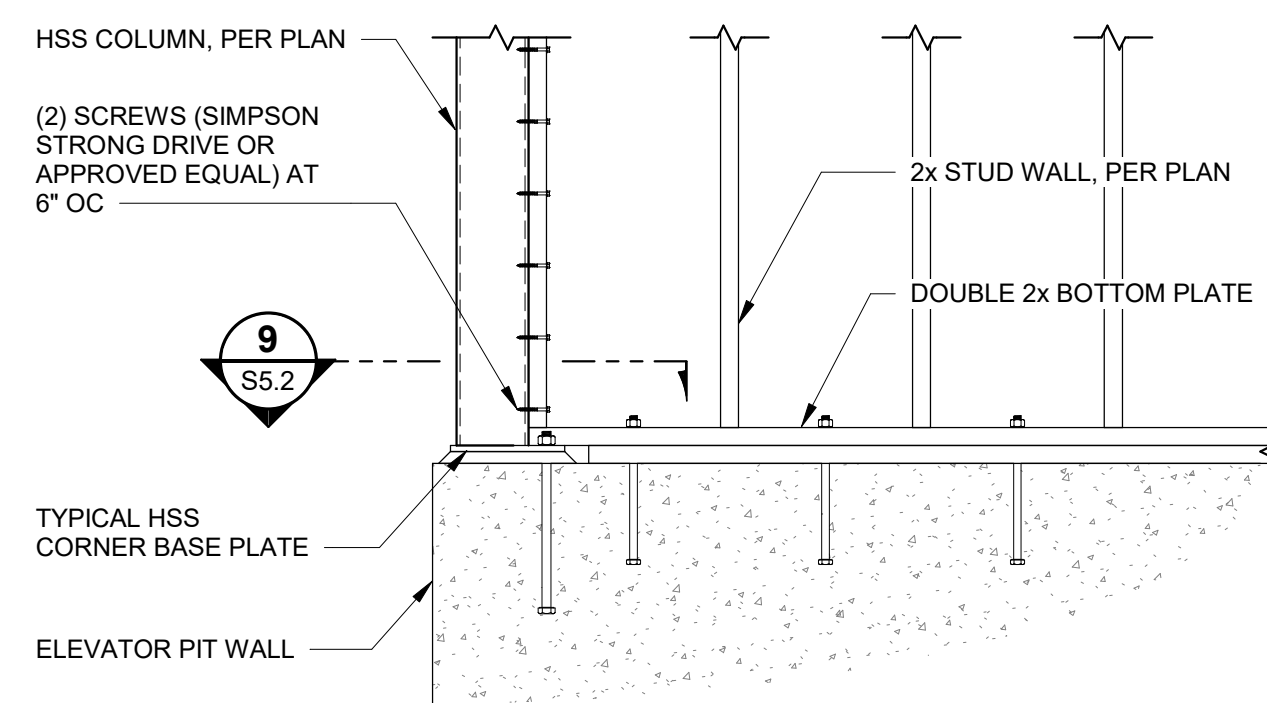
**9** ELEVATOR COLUMN  
S5.2 SCALE: 3/4" = 1'-0"



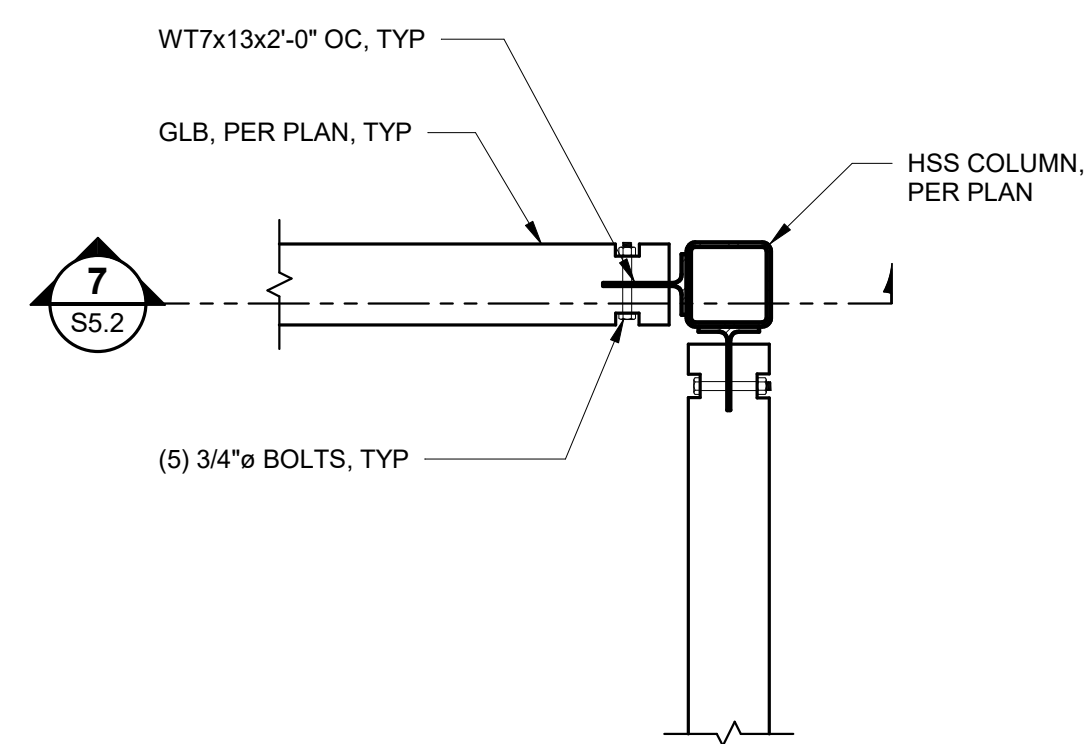
**2** ELEVATOR COLUMN AT ROOF  
S5.2 SCALE: 3/4" = 1'-0"



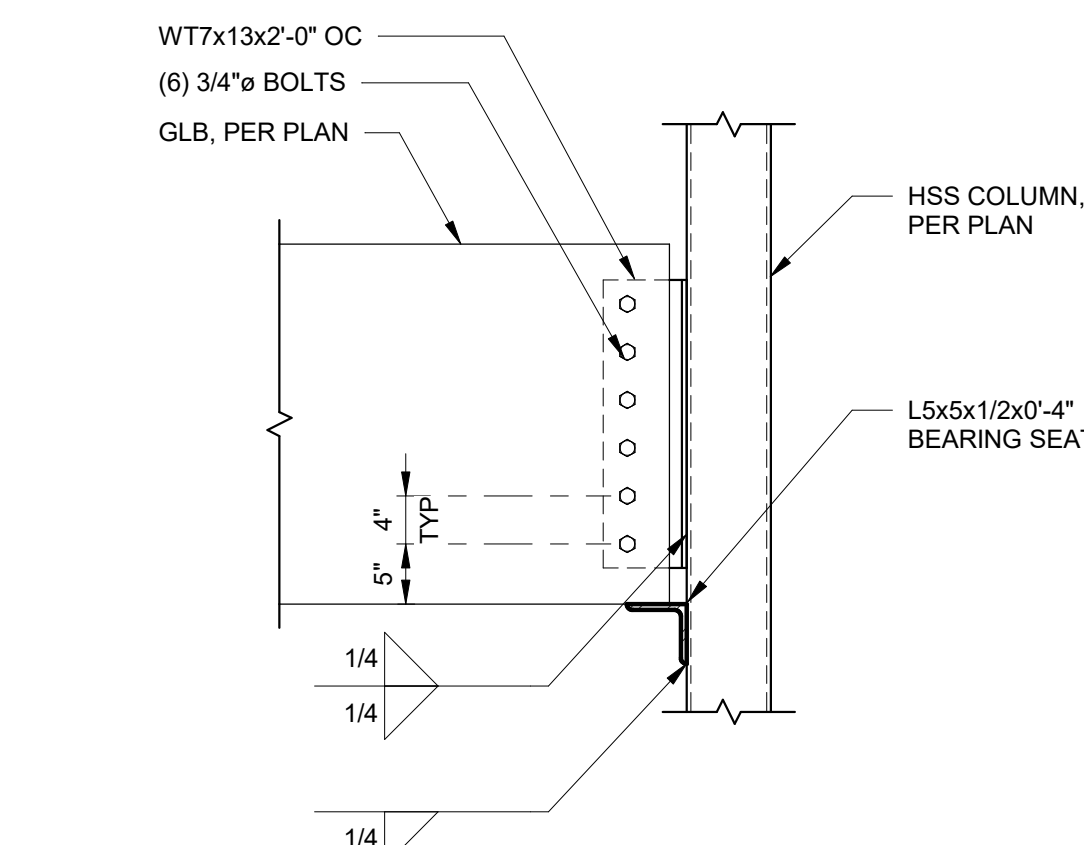
**6** ELEVATOR COLUMN AT SECOND FLOOR  
S5.2 SCALE: 3/4" = 1'-0"



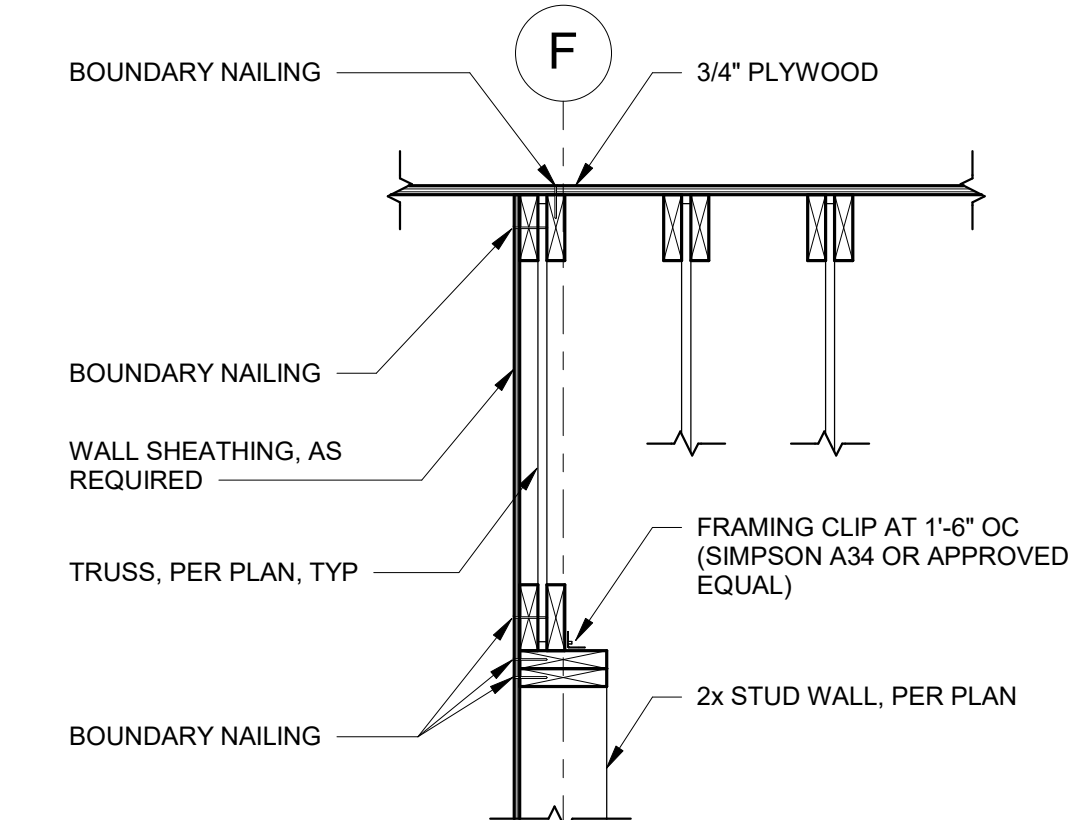
**10** ELEVATOR COLUMN AT FIRST FLOOR  
S5.2 SCALE: 3/4" = 1'-0"



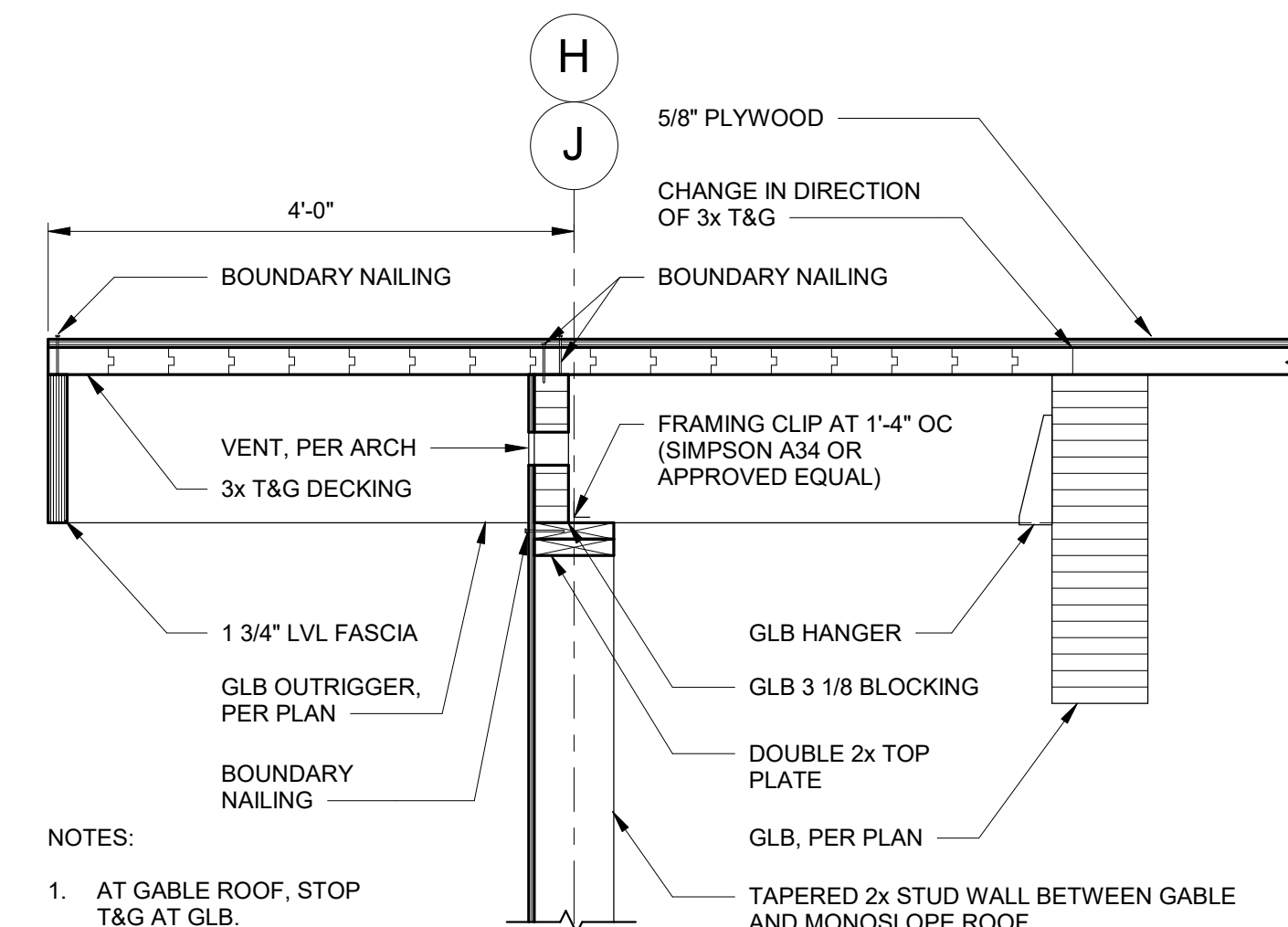
**3** GLB TO COLUMN AT CORNER  
S5.2 SCALE: 3/4" = 1'-0"



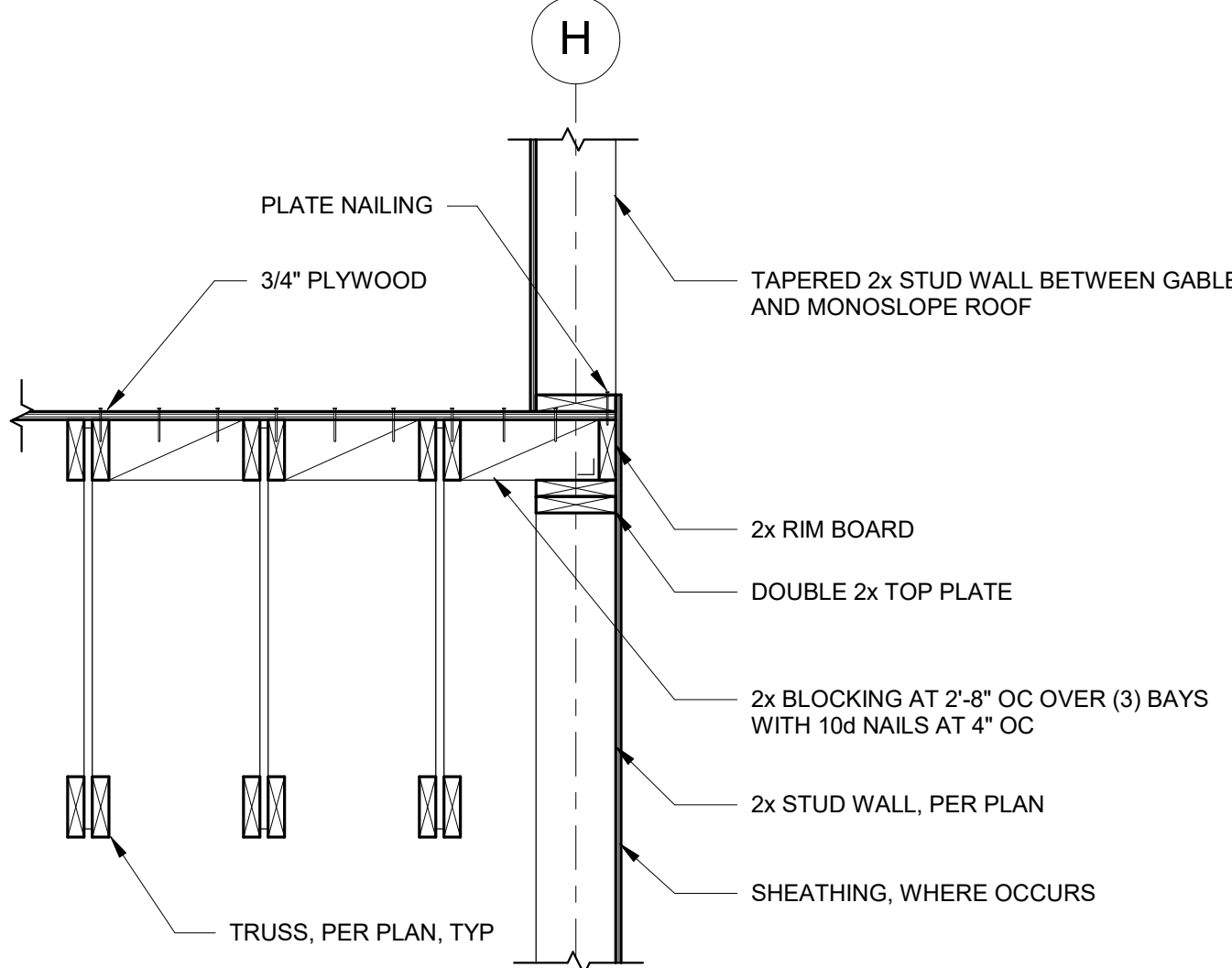
**7** GLB TO COLUMN TEE AT MID-HEIGHT  
S5.2 SCALE: 3/4" = 1'-0"



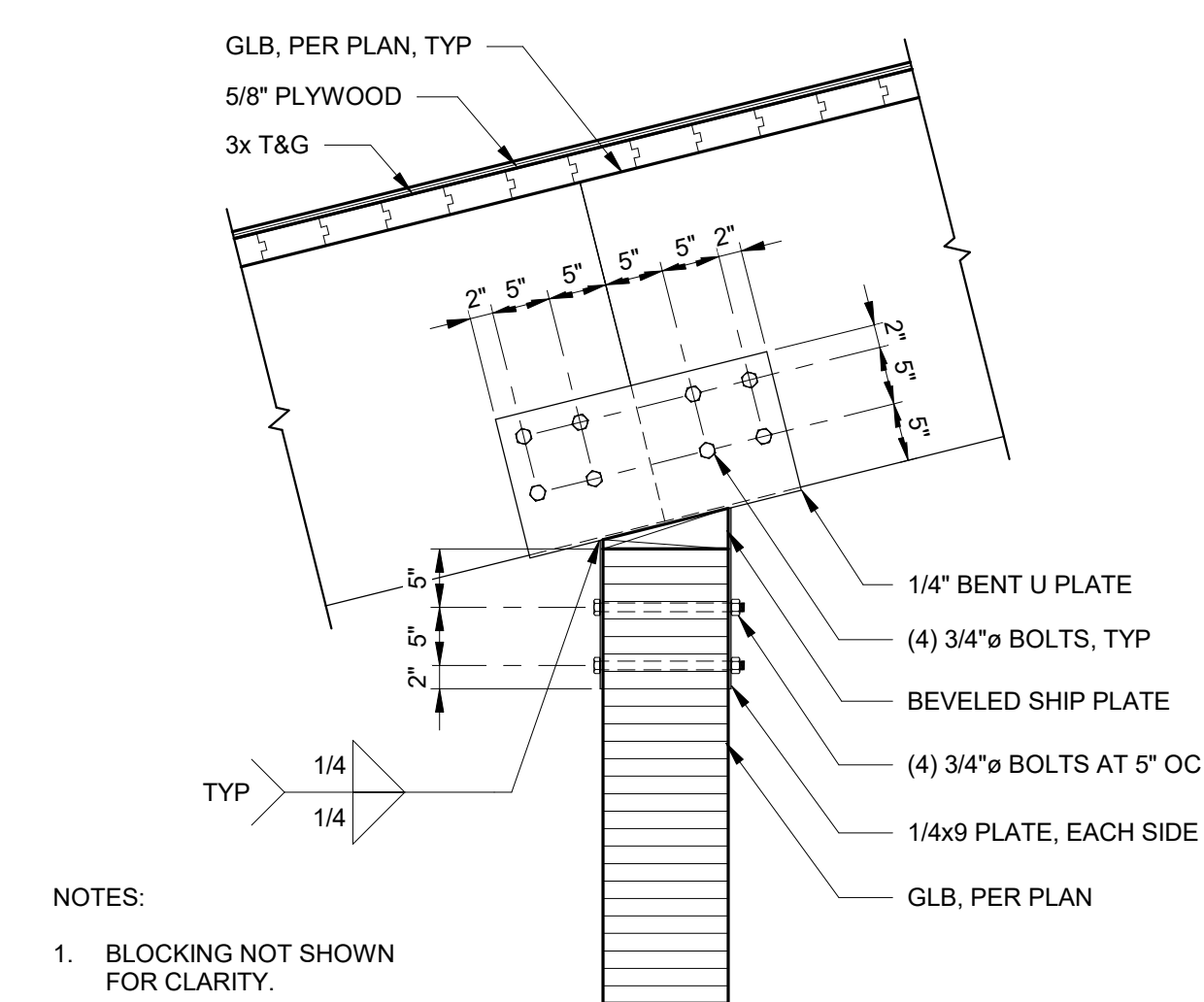
**11** BEARING WALL PARALLEL TO TRUSS  
S5.2 SCALE: 3/4" = 1'-0"



**4** TYPICAL ROOF RAKE  
S5.2 SCALE: 3/4" = 1'-0"



**8** GABLE ROOF AT GRID LINE H  
S5.2 SCALE: 3/4" = 1'-0"



**12** BEARING WALL PARALLEL TO TRUSS  
S5.2 SCALE: 3/4" = 1'-0"

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Sheet Contents  
FRAMING SECTIONS AND  
DETAILS (2 OF 4)

Sheet No.  
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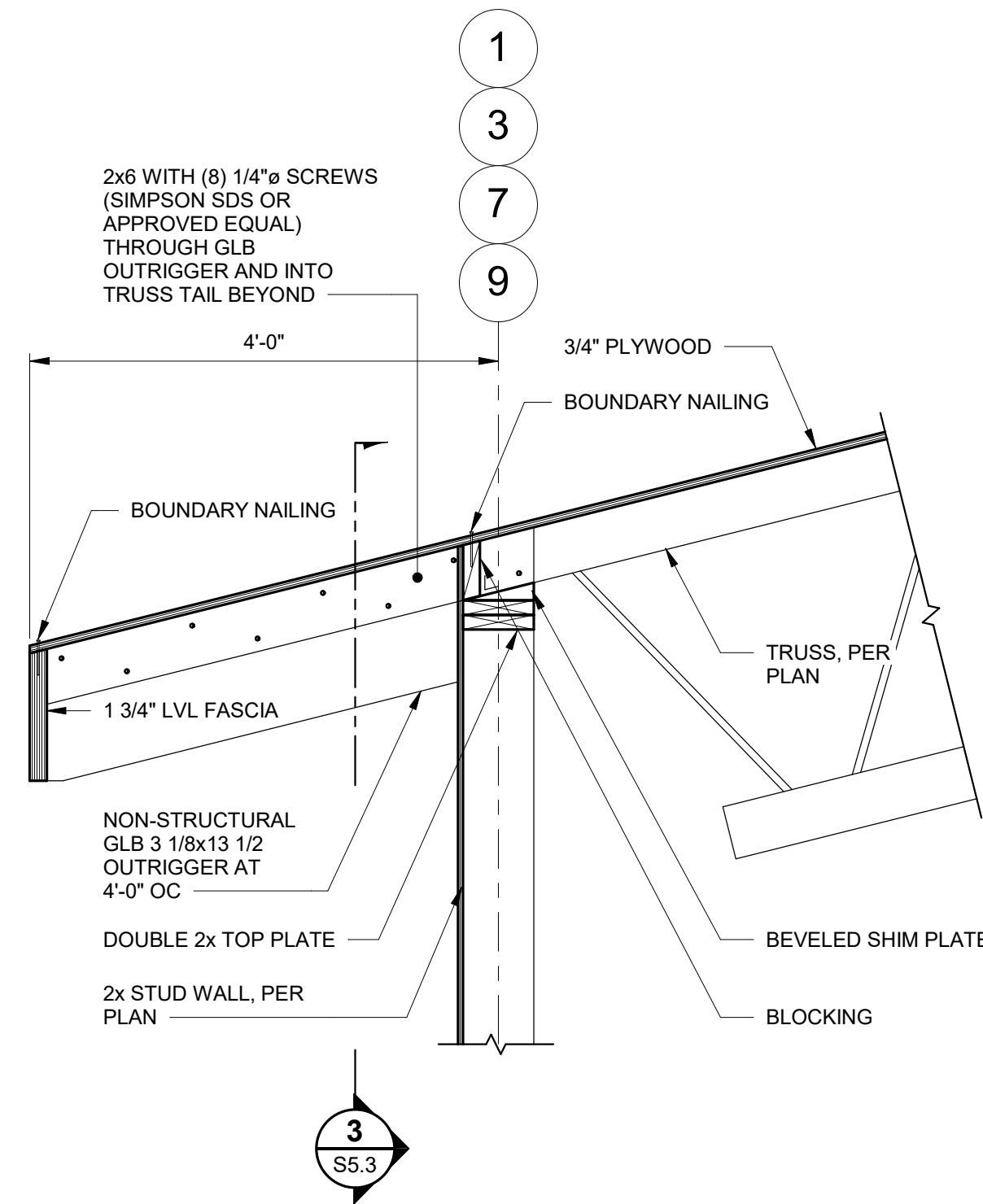
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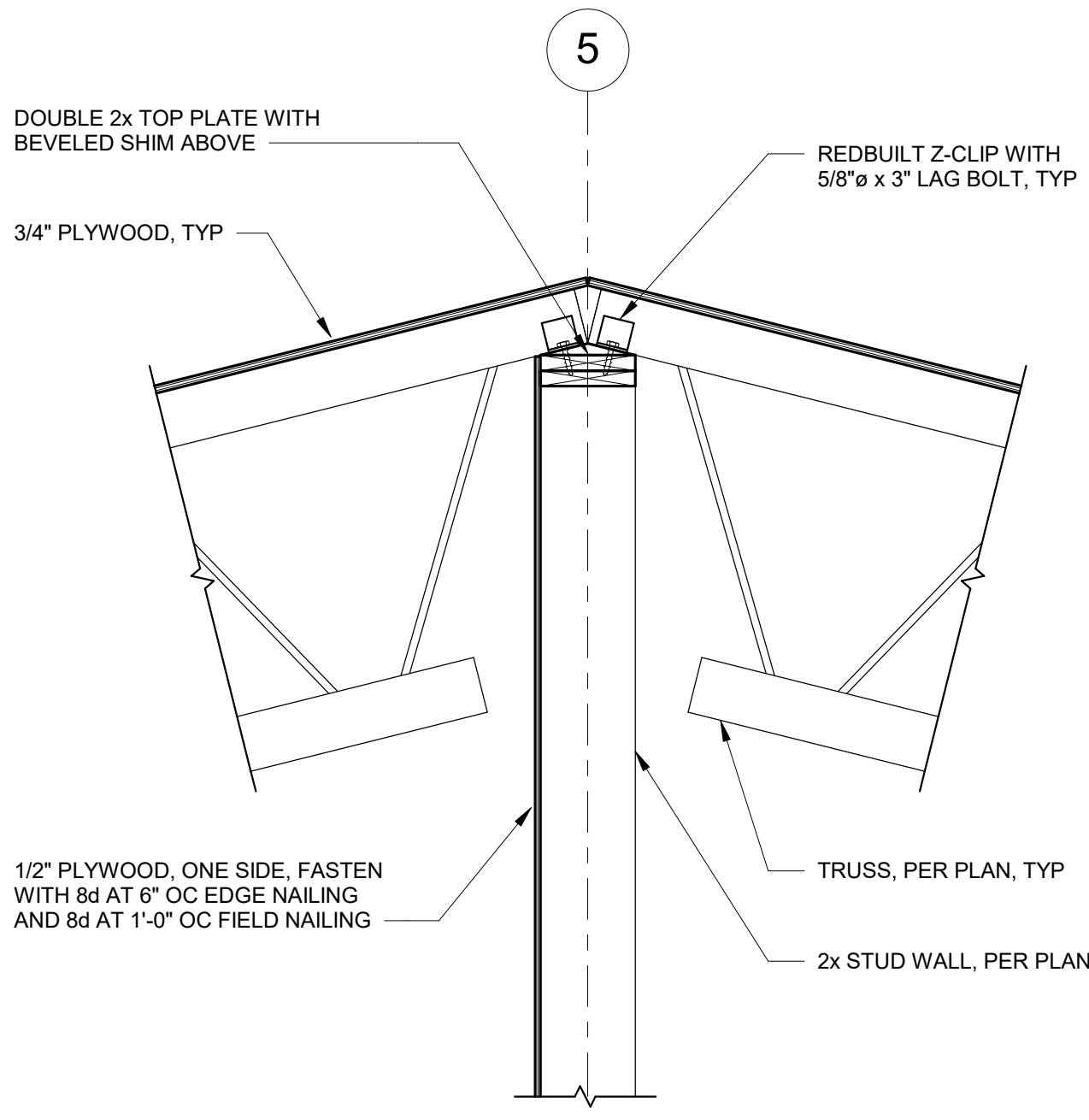
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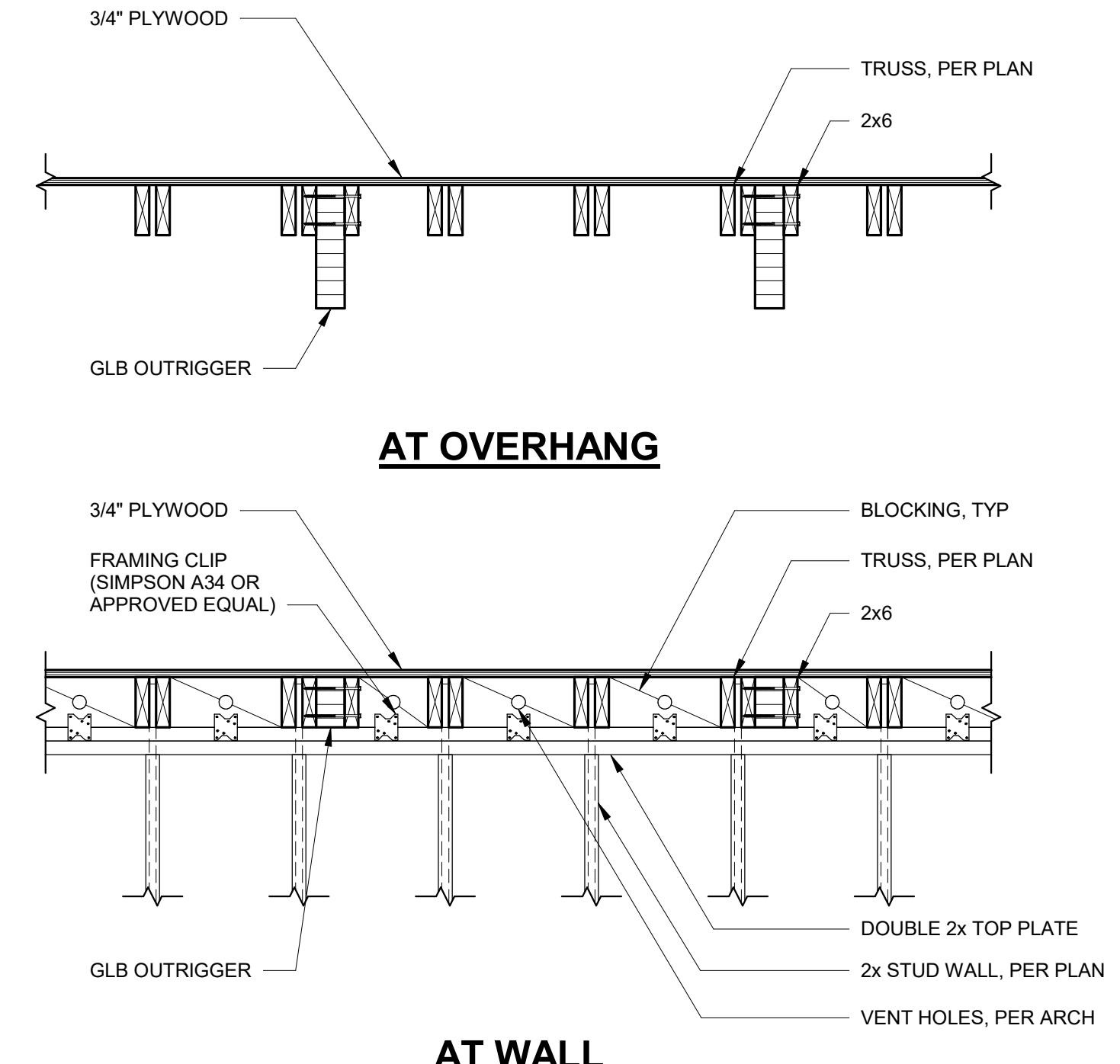
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**1 EAVE AT GABLE ROOF**  
S5.3 SCALE: 3/4" = 1'-0"

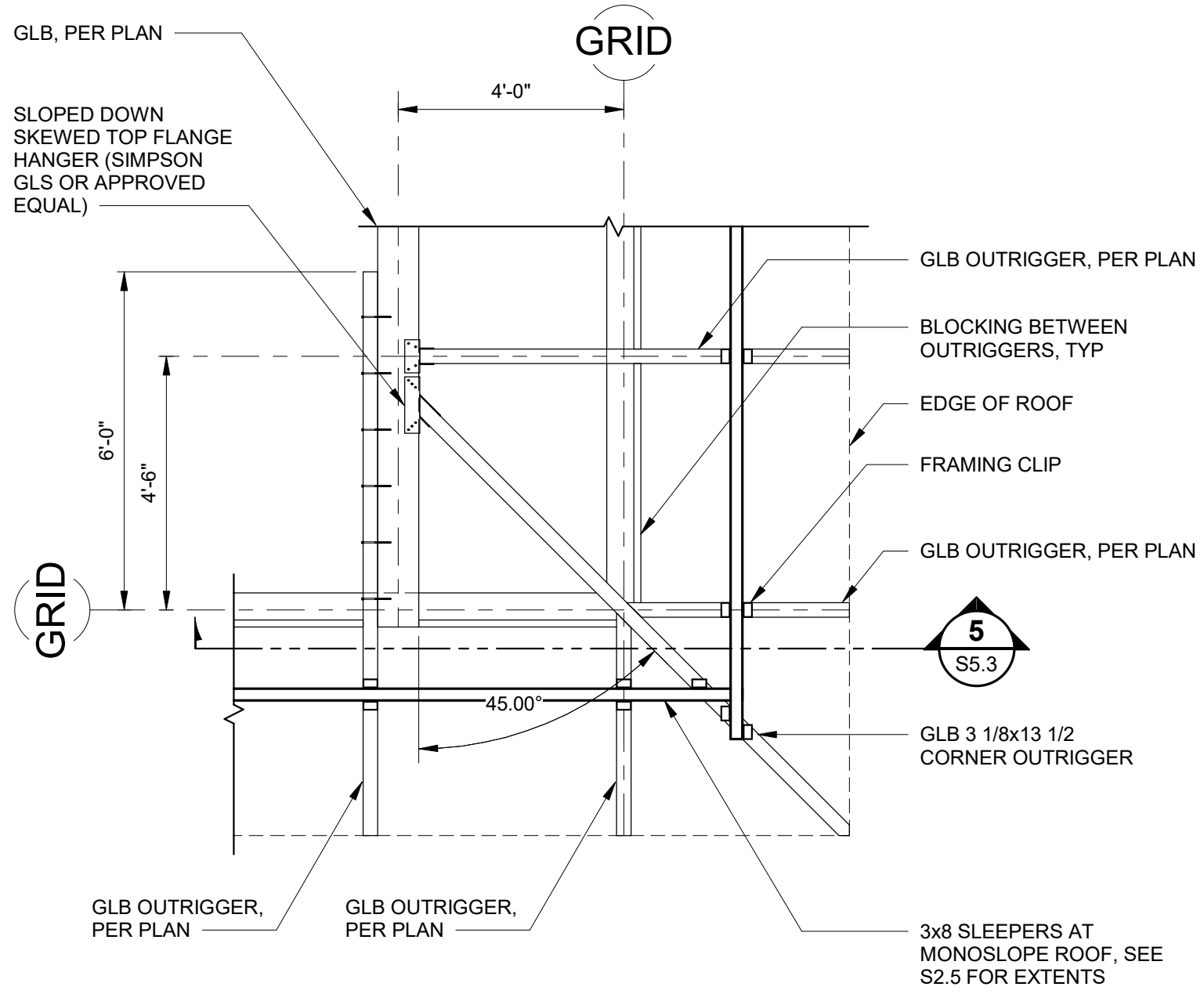


**2 RIDGE AT GABLE ROOF**  
S5.3 SCALE: 3/4" = 1'-0"

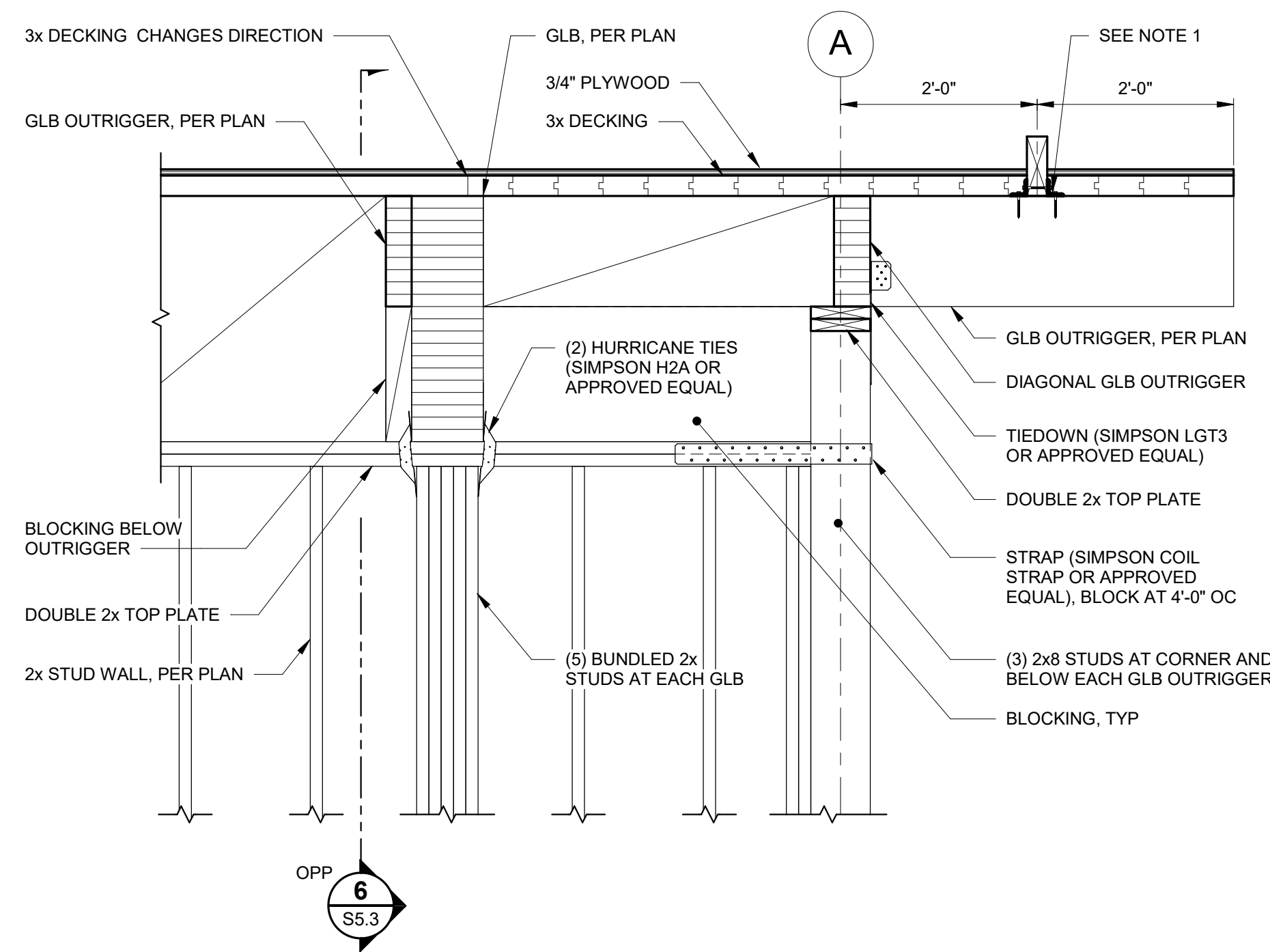


**3 EAVE AT GABLE ROOF**  
S5.3 SCALE: 3/4" = 1'-0"

NOTES:  
1. SEE ARCHITECTURAL FOR SOFFIT.

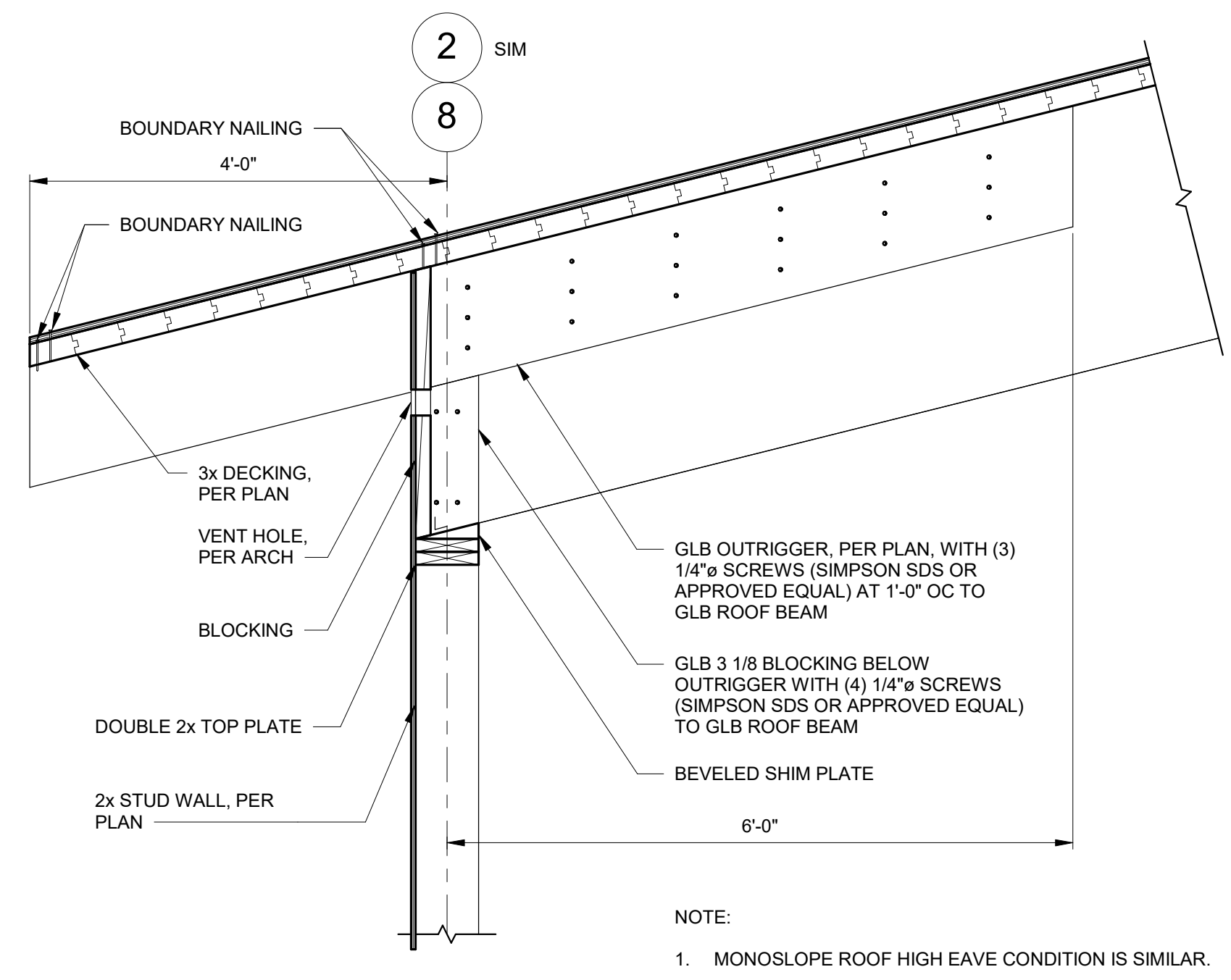


**4 ROOF CORNER OUTRIGGING**  
S5.3 SCALE: 3/8" = 1'-0"



**5 CORNER EAVE AT MONOSLOPE ROOF**  
S5.3 SCALE: 3/4" = 1'-0"

NOTES:  
1. 3x8 SLEEPER. LOCATE PER PLAN. PROVIDE (1) L2x2x1/4x0'-3" EACH SIDE WITH (2) 1/4"Ø x 2 1/2" SCREWS INTO SLEEPER AND (2) 1/4"Ø x 3" SCREWS INTO GLB OUTRIGGER BELOW. SCREWS TO BE SIMPSON SDS OR APPROVED EQUAL.



**6 EAVE AT MONOSLOPE ROOF**  
S5.3 SCALE: 3/4" = 1'-0"

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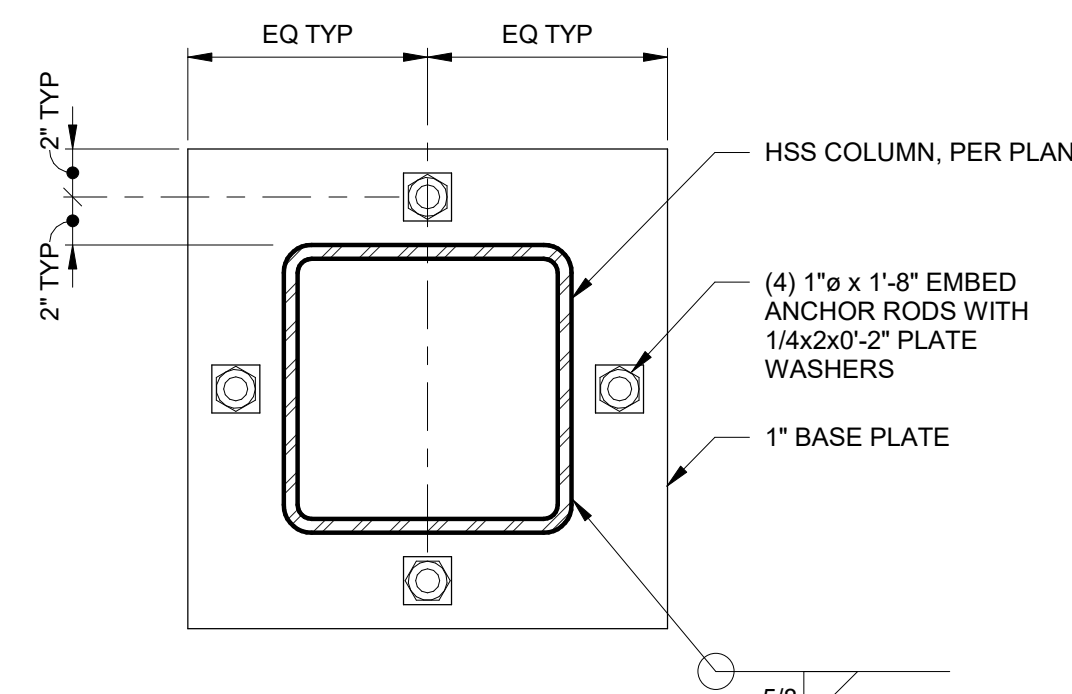
No.	Description	Date

Drawn by DJM	Date 06/18/2019
Checked AKM	Job No. 17341JN

Sheet Contents  
FRAMING SECTIONS AND  
DETAILS (3 OF 4)

Sheet No.  
**S5.3**

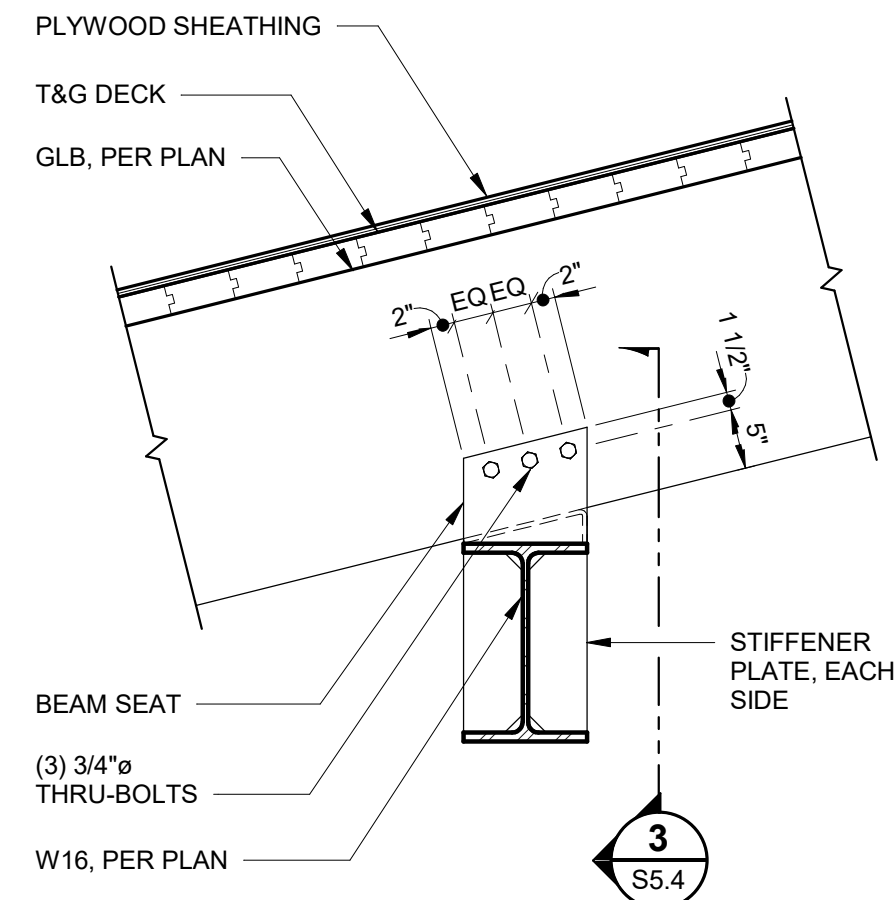




NOTES:  
1. PROVIDE 1" NON-SHRINK GROUT BELOW BASE PLATE.

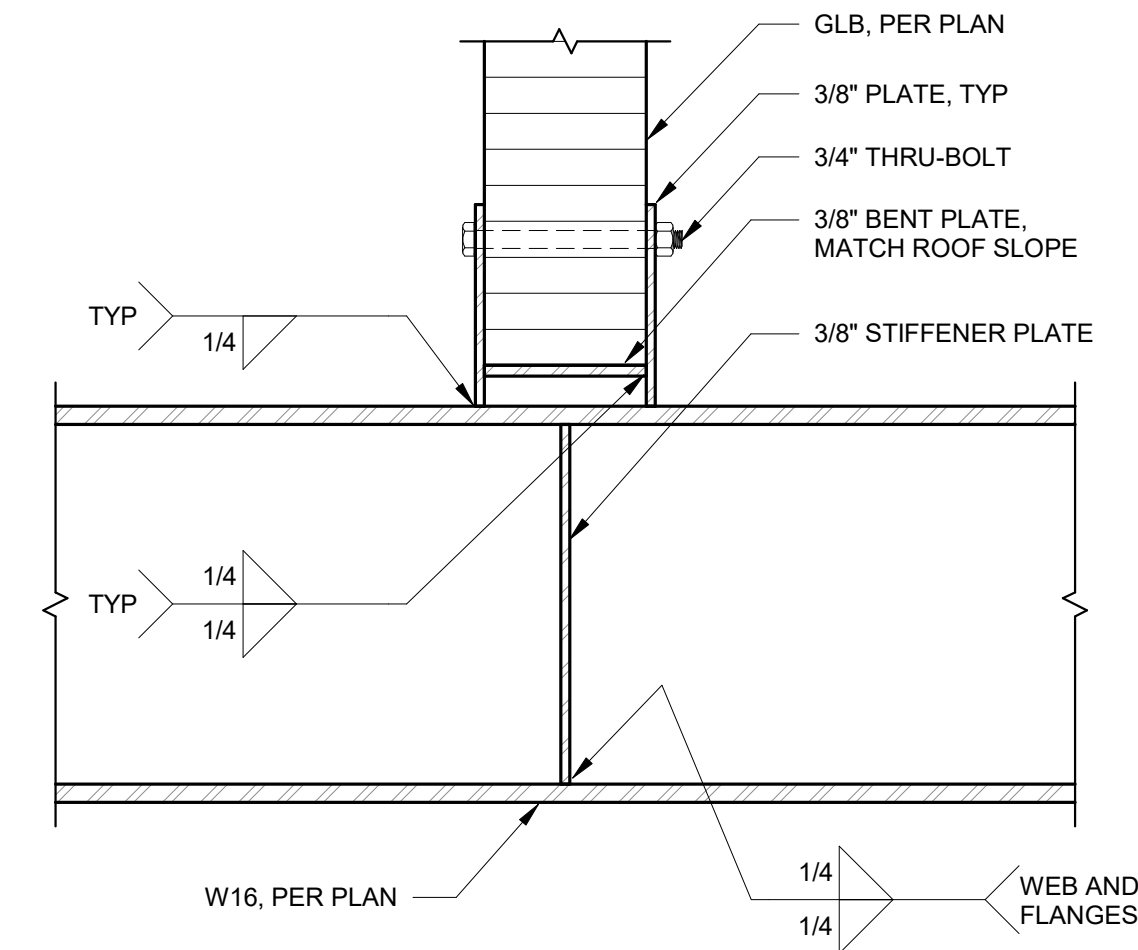
**1 CANOPY BASE PLATE DETAIL**

S5.4 SCALE: 1 1/2" = 1'-0"



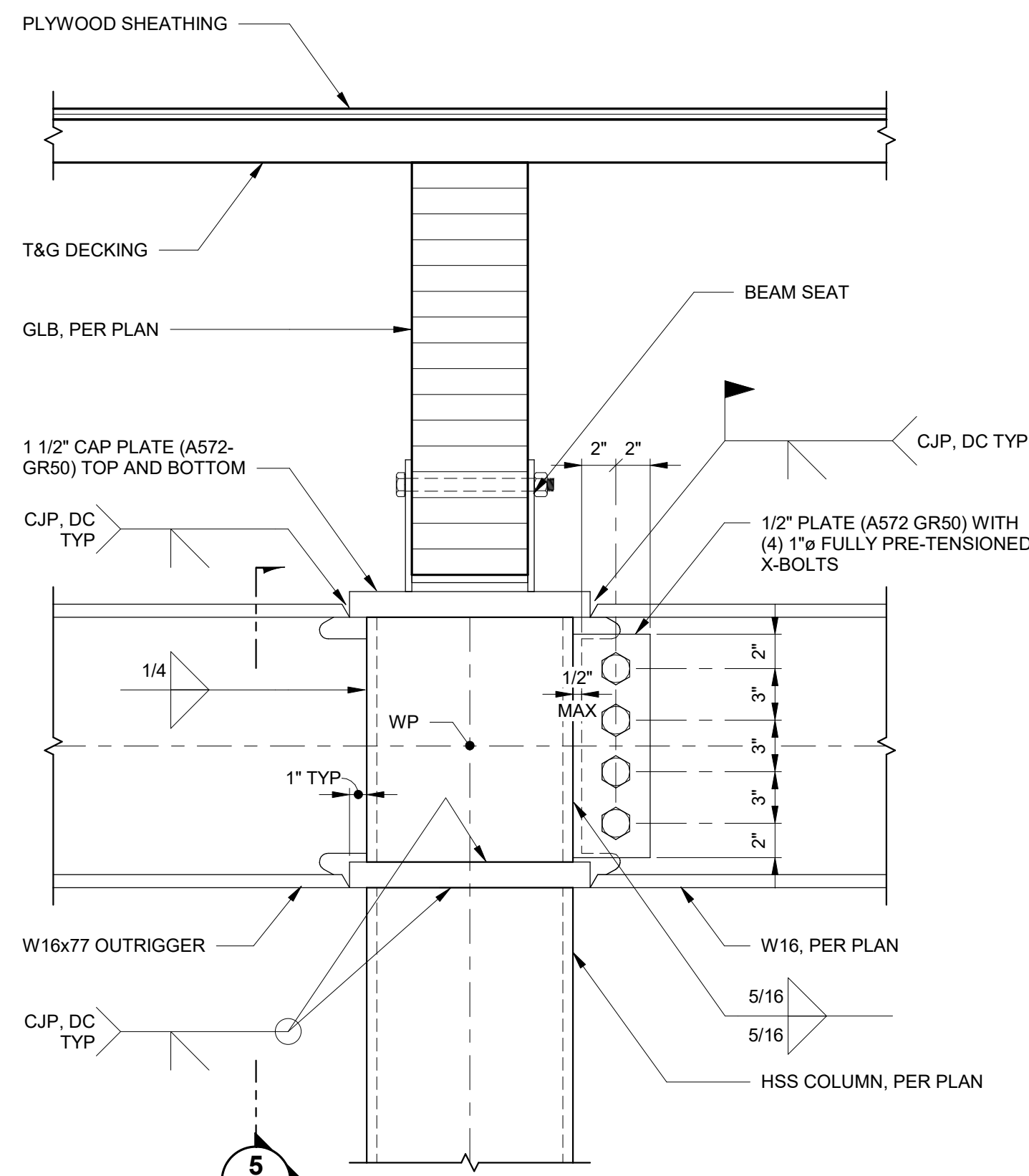
**2 CANOPY FRAMING DETAIL**

S5.4 SCALE: 3/4" = 1'-0"



**3 CANOPY FRAMING DETAIL**

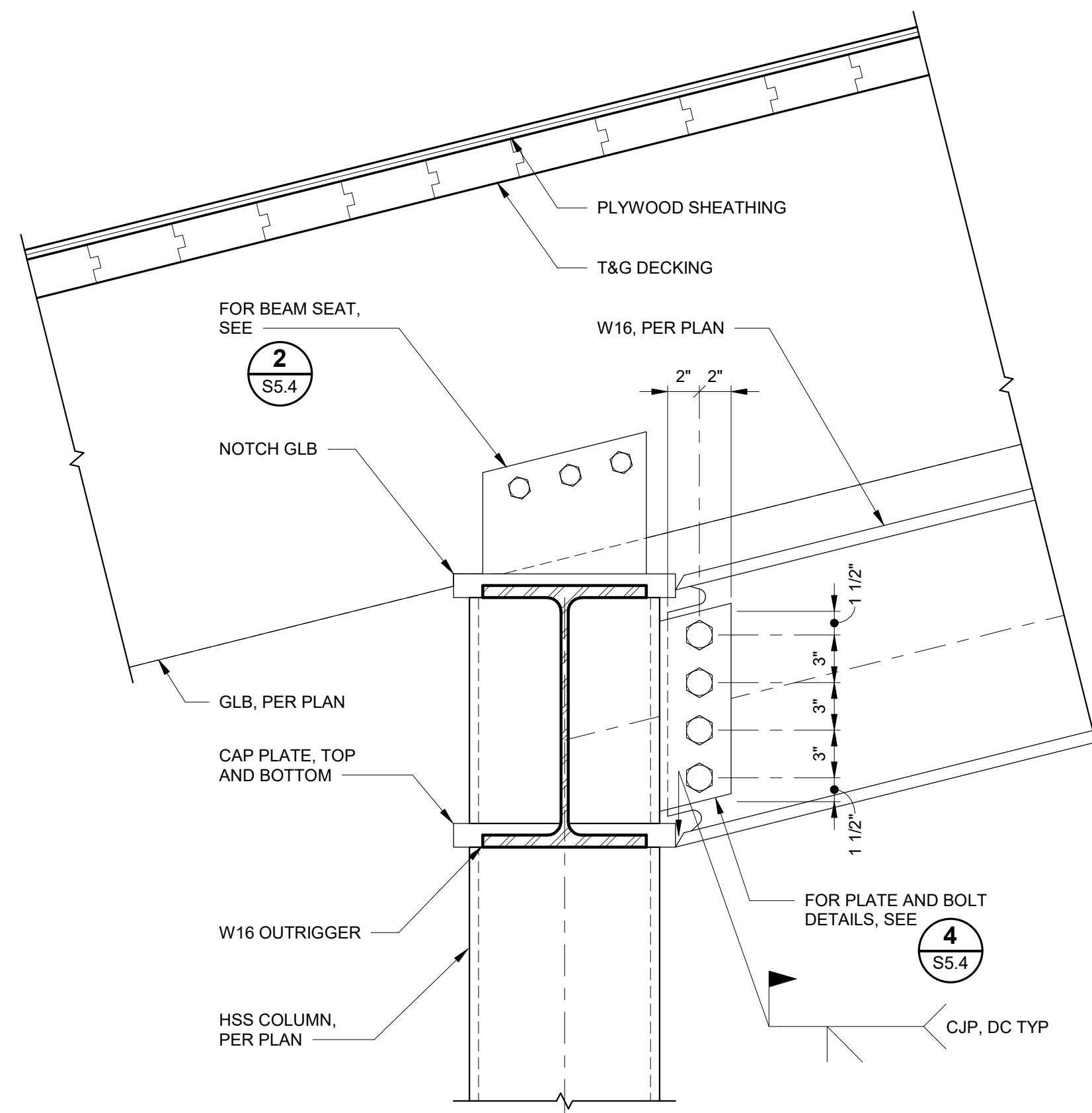
S5.4 SCALE: 1 1/2" = 1'-0"



NOTES:  
1. REMOVE BACKING AT BEAM TO COLUMN CJP WELDS. BACKGOUGE TO SOUND WELD METAL AND REINFORCE WITH A 5/16" FILLET WELD.

**4 CANOPY MOMENT FRAME DETAIL**

S5.4 SCALE: 1 1/2" = 1'-0"



**5 CANOPY MOMENT FRAME DETAIL**

S5.4 SCALE: 1 1/2" = 1'-0"

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Checked AKM	Job No. 17341JN

Sheet Contents  
FRAMING SECTIONS AND  
DETAILS (4 OF 4)

Sheet No.  
**S5.4**